## 2015-2019 CRASH DATA

CRASHES


INJURIES


Opinion of probable cost for single-lane roundabout

## \$1.39 MILLION

## L. Robert T. Longway Boulevard and Walnut Street

The intersection of Robert T. Longway Boulevard and Walnut Street was included in the early preliminary engineering phase with support from of the City of Flint due to intersection operations and future plans to close Kearlsey Street at Walnut and make the intersection at Robert T. Longway Boulevard and Walnut Street the eastern entrance into the Flint Cultural Center. This intersection is a secondary analysis Tier Three intersection.

During the skim analysis, it was observed that there were two angle crashes and zero head on left turn crashes over the 5-year period. These crashes resulted in no fatalities or type A, type B, or type C injuries at the intersection.

## Future No-Build Conditions

The intersection of Robert T. Longway and Walnut Street is a two-way stop control intersection. Robert T. Longway runs east/west and is a four-lane roadway with two lanes in each direction. Walnut Street runs north/south and is a two-lane road with one lane in each direction. The Walnut Street approaches are stop controlled.

In the southwest quadrant of the intersection is a Consumers Energy electrical sub-station. The Buick Gallery is located adjacent to the right-of-way in the southeast quadrant. To the north of the intersection is vacant property owned by the City of Flint but there are high-voltage power lines that run east/west overhead.

An aerial of the existing intersection can be seen in Figure 13: Aerial view of Robert T. Longway and Walnut Street.

Figure 13: Aerial view of Robert T. Longway and Walnut Street


An operational analysis of the no-build condition was completed for the intersection using the 2045 forecast traffic volumes. The results of the analysis for future no-build conditions reveals that all approaches and movements of the intersection operate at LOS C or better during the AM and PM peak hours.

The $95^{\text {th }}$ percentile queue lengths were reviewed at the intersection and results showed that all approaches experienced a maximum queue length of 48 feet ( 2 vehicles) during the AM peak hour and 44 feet ( 2 vehicles) during the PM peak hour.

The operational results for future no-build conditions are presented in Table 28.

Table 28: Operational Analysis for 2045 No-Build Conditions

| Intersection | Approach | AM Peak |  | PM Peak |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Delay/LOS | Queue (veh)* | Delay/LOS | Queue (veh)* |
| Robert T. Longway Boulevard <br> and Walnut Street | Eastbound | $0.2 / \mathrm{A}$ | $1(17 \mathrm{ft})$ | $0.1 / \mathrm{A}$ | $1(32 \mathrm{ft})$ |
|  | Westbound | $0.1 / \mathrm{A}$ | $1(14 \mathrm{ft})$ | $0.1 / \mathrm{A}$ | $1(16 \mathrm{ft})$ |
|  | Northbound | $12.7 / \mathrm{B}$ | $2(48 \mathrm{ft})$ | $18.2 / \mathrm{C}$ | $2(41 \mathrm{ft})$ |
|  | Southbound | $12.5 / \mathrm{B}$ | $2(38 \mathrm{ft})$ | $17.6 / \mathrm{C}$ | $2(44 \mathrm{ft})$ |
|  | Overall | $1.1 / \mathrm{A}$ |  | $1.0 / \mathrm{A}$ |  |

* $95^{\text {th }}$ percentile queue length


## Roundabout Conditions

Due to the current four-lane cross-section on Robert T. Longway Boulevard, two potential roundabout layouts were developed. A two-lane by one-lane roundabout was drawn that allowed for a two-lane
approach for eastbound and westbound Robert T. Longway and a single-lane approach for Walnut Street. Because of the location of both the Consumers Energy electric substation and the Buick Gallery, the roundabout was offset to the north of the current location. The property to the north of the intersection is owned by the City of Flint, therefore no additional right-of-way would be needed to accommodate this shift. Although no additional right-of-way will be necessary, the Consumers Energy easement and overhead power will need to be relocated to accommodate a proposed roundabout. A full layout can be found at the end of this section.

An operational analysis of the roundabout (build) condition was completed for the intersection using the 2045 forecast traffic volumes. The results of the analysis for roundabout (build) conditions reveals that all approaches and movements of the intersection operate at LOS A during the AM and PM peak hours.

The $95^{\text {th }}$ percentile queue lengths were reviewed at the intersection and results showed that all approaches experienced a maximum queue length of one vehicle during the AM peak hour and PM peak hour for the 2 by 1 layout and 2 vehicles during the AM and PM peak hour for the single-lane roundabout.

The operational results for future roundabout (build) conditions are presented in Table 29.

Table 29: Operational Analysis for Future Conditions

| Intersection | Approach | AM Peak |  | PM Peak |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Delay/LOS | Queue (veh)* | Delay/LOS | Queue (veh)* |
| Robert T. Longway Blvd and <br> Walnut Street (2-EB/WB, 1- <br> NB/SB circulating) | Eastbound | $3.6 / \mathrm{A}$ | 0.6 | $4.1 / \mathrm{A}$ | 0.9 |
|  | Westbound | $3.3 / \mathrm{A}$ | 0.5 | $3.9 / \mathrm{A}$ | 0.8 |
|  | Northbound | $3.6 / \mathrm{A}$ | 0.1 | $3.8 / \mathrm{A}$ | 0.1 |
|  | Southbound | $3.0 / \mathrm{A}$ | 0.0 | $3.5 / \mathrm{A}$ | 0.1 |
|  | Overall | $3.5 / \mathrm{A}$ |  | $4.0 / \mathrm{A}$ |  |
| Robert T. Longway Blvd and <br> Walnut Street (Single-lane RAB) | Eastbound | $4.3 / \mathrm{A}$ | 1.4 | $5.1 / \mathrm{A}$ | 2.3 |
|  | Westbound | $4.1 / \mathrm{A}$ | 1.2 | $4.8 / \mathrm{A}$ | 2.0 |
|  | Northbound | $3.7 / \mathrm{A}$ | 0.1 | $4.1 / \mathrm{A}$ | 0.1 |
|  | Southbound | $3.5 / \mathrm{A}$ | 0.0 | $3.8 / \mathrm{A}$ | 0.1 |
|  | Overall | $4.2 / \mathrm{A}$ |  | $4.9 / \mathrm{A}$ |  |

* $95^{\text {th }}$ percentile queue length

Opinions of probable costs were developed for both scenarios. The 2 by 1 roundabout probable cost is $\$ 1.61$ million in year 2025 dollars. The single-lane roundabout probable cost is $\$ 1.39$ million in year 2025 dollars. These probable costs include a 20 percent contingency and 3 percent annual inflation. The potential fees from Consumers Energy to relocate their high voltage poles to accommodate the proposed layouts was estimated to be $\$ 200,000$ based on information provided from Consumers Energy. A full breakdown along with all assumptions can be found in Appendix 3.

Potential funding for this improvement could be made possible by regular road improvement funding or an earmark. There are no significant crashes of the type that would make the intersections eligible for safety funds and the intersection does not experience enough delay to make it eligible for CMAQ funding.

## The Whiting Special Event Traffic Analysis

A separate traffic analysis was completed to determine the feasibility of a roundabout with an event at The Whiting. A trip generation was performed for The Whiting based on a 2,043 -seat venue. A $50 / 50$ split was used for distribution of vehicles leaving using Kearsley Street and Walnut Street. It was determined based on a trip generation rate from the $9^{\text {th }}$ Edition of the Trip Generation Manual, there would be a total of 41 trips ( $21 \mathrm{in}, 20$ out) generated during the PM peak hour. Table 30 presents the trip generation.

Table 30: The Whiting Trip Generation

|  | Land Use | Land Use Code | Units |  | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Development |  |  |  |  | In | Out | Total | In | Out | Total |
| The Whiting | Live Theater | 441 | 2,043 | seats | - | - | - | 21 | 20 | 41 |

This results in roughly five to six vehicles generated using Walnut Street to enter/exit The Whiting. Special events typically do not occur during the typical peak hours (7-9 a.m. and 4-6 p.m.). To create a more conservative case, 200 vehicles were added to the major movements entering/exiting The Whiting at Robert T. Longway Boulevard and Walnut Street: northbound right and left turning movements, eastbound right turning movement, westbound left turning movement. Table 31 presents the operational results for a single lane roundabout with a special event at The Whiting.

Table 31: Operational Analysis for Future Conditions - The Whiting Special Event

| Intersection | Approach | Event Peak |  |
| :--- | :--- | :---: | :---: |
|  |  | Delay/LOS | Queue (veh)* |
| Robert T. Longway Blvd and <br> Walnut Street (Single Lane RAB) | Eastbound | $8.3 / \mathrm{A}$ | 5.4 |
|  | Westbound | $7.5 / \mathrm{A}$ | 4.6 |
|  | Northbound | $6.9 / \mathrm{A}$ | 2.5 |
|  | Southbound | $4.9 / \mathrm{A}$ | 0.1 |
|  | Overall | 7.6/A |  |

*95 ${ }^{\text {th }}$ Percentile queue length

Based on the addition of event traffic, a proposed single lane roundabout will operate with acceptable LOS during an event at The Whiting.

## Recommendation

A roundabout would be feasible at this location, although it would be quite costly due to the necessary relocation of the Consumers Energy power lines. If the City of Flint moves forward with a road diet on Robert T. Longway east of northbound Chavez, a single-lane roundabout would be desirable.



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US
All Movements
ID: 817412, Location: 43.025598, -83.676457

| Leg <br> Direction | Robert T Longway Blvd Eastbound |  |  |  | Robert T Longway Blvd Westbound |  |  |  |  |  | Walnut St Northbound |  |  |  |  |  | Walnut St Southbound |  |  |  |  |  | Int |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L T | R U | App | Ped* | L | T | R | U | App | ed* | L | T | R | U | App | Ped* | L | T | R | U | App |  |  |
| 2021-03-09 7:00AM | 033 | 40 | 37 | 0 | 2 | 24 | 0 | 0 | 26 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 65 |
| 7:15AM | 135 | 40 | 40 | 0 | 0 | 39 | 0 | 0 | 39 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 82 |
| 7:30AM | 166 | 80 | 75 | 0 | 0 | 73 | 0 | 0 | 73 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 154 |
| 7:45AM | 070 | 80 | 78 | 0 | 2 | 73 | 1 | 0 | 76 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 156 |
| Hourly Total | 2204 | 240 | 230 | 0 | 4 | 209 | 1 | 0 | 214 | 0 | 1 | 0 | 3 | 0 | 4 | 0 | 5 | 0 | 4 | 0 | 9 | 0 | 457 |
| 8:00AM | 272 | 60 | 80 | 0 | 1 | 64 | 0 | 0 | 65 | 0 | 7 | 0 | 7 | 0 | 14 | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 162 |
| 8:15AM | 363 | 40 | 70 | 0 | 1 | 46 | 0 | 0 | 47 | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 125 |
| 8:30AM | 066 | 80 | 74 | 0 | 0 | 55 | 1 | 0 | 56 | 0 | 2 | 0 | 1 | 0 | 3 | 0 | 0 | 0 | 5 | 0 | 5 | 0 | 138 |
| 8:45AM | 060 | 120 | 72 | 0 | 0 | 57 | 0 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 0 | 132 |
| Hourly Total | $5 \quad 261$ | 300 | 296 | 0 | 2 | 222 | 1 | 0 | 225 | 0 | 11 | 0 | 12 | 0 | 23 | 0 | 2 | 0 | 11 | 0 | 13 | 0 | 557 |
| 4:00PM | 297 | 40 | 103 | 0 | 1 | 110 | 1 | 0 | 112 | 0 | 1 | 0 | 3 | 0 | 4 | 0 | 3 | 1 | 2 | 0 | 6 | 0 | 225 |
| 4:15PM | 299 | 20 | 103 | 0 | 1 | 71 | 0 | 0 | 72 | 0 | 1 | 2 | 2 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 182 |
| 4:30PM | 190 | 10 | 92 | 0 | 1 | 107 | 2 | 0 | 110 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 2 | 2 | 2 | 0 | 6 | 0 | 211 |
| 4:45PM | 0102 | 50 | 107 | 0 | 1 | 81 | 1 | 0 | 83 | 0 | 3 | 0 | 1 | 0 | 4 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 196 |
| Hourly Total | 5388 | 120 | 405 | 0 | 4 | 369 | 4 | 0 | 377 | 0 | 8 | 2 | 6 | 0 | 16 | 0 | 6 | 3 | 7 | 0 | 16 | 0 | 814 |
| 5:00PM | 292 | 10 0 | 104 | 0 | 0 | 85 | 1 | 0 | 86 | 0 | 4 | 0 | 4 | 0 | 8 | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 202 |
| 5:15PM | 181 | $9 \quad 0$ | 91 | 0 | 2 | 71 | 0 | 0 | 73 | 0 | 3 | 0 | 2 | 0 | 5 | 0 | 1 | 1 | 4 | 0 | 6 | 0 | 175 |
| 5:30PM | 496 | 70 | 107 | 0 | 3 | 80 | 0 | 0 | 83 | 0 | 1 | 2 | 2 | 0 | 5 | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 199 |
| 5:45PM | 377 | 70 | 87 | 1 | 0 | 68 | 2 | 0 | 70 | 0 | 0 | 1 | 2 | 0 | 3 | 0 | 2 | 0 | 3 | 0 | 5 | 0 | 165 |
| Hourly Total | 10346 | 330 | 389 | 1 | 5 | 304 | 3 | 0 | 312 | 0 | 8 | 3 | 10 | 0 | 21 | 0 | 7 | 1 | 11 | 0 | 19 | 0 | 741 |
| Total | 221199 | $99 \quad 0$ | 1320 | 1 | 15 | 1104 | 9 | 0 | 1128 | 0 | 28 | 5 | 31 | 0 | 64 | 0 | 20 | 4 | 33 | 0 | 57 | 0 | 2569 |
| \% Approach | 1.7\% 90.8\% | 7.5\% 0\% | - | - | 1.3\% | 97.9\% | 0.8\% 0\% |  | - |  | 43.8\% | 7.8\% | 48.4\% 0\% |  | - |  | 35.1\% | 7.0\% | 57.9\% 0\% |  | - |  |  |
| \% Total | 0.9\% 46.7\% | 3.9\% 0\% 5 | 51.4\% | - | 0.6\% | 43.0\% | 0.4\% 0\% | \% 4 | 43.9\% |  | 1.1\% | 0.2\% | 1.2\% 0\% |  | 2.5\% |  | 0.8\% | 0.2\% | 1.3\% 0\% |  | 2.2\% |  |  |
| Lights | $22 \quad 1167$ | $94 \quad 0$ | 1283 | - | 14 | 1078 | 9 | 0 | 1101 | - | 24 | 5 | 31 | 0 | 60 |  | 20 | 4 | 32 | 0 | 56 | - | 2500 |
| \% Lights | 100\% 97.3\% | 94.9\% 0\% 9 | 97.2\% |  | 93.3\% 9 | 97.6\% | 100\% 0\% | \% 9 | 97.6\% |  | 85.7\% 1 | 100\% | 100\% 0\% | \% 9 | 93.8\% |  | 100\% | 100\% | 97.0\% 0\% | \% 9 | 98.2\% |  | 97.3\% |
| Single-Unit Trucks | $0 \quad 17$ | 10 | 18 | - | 1 | 15 | 0 | 0 | 16 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 34 |
| \% Single-Unit Trucks | 0\% 1.4\% | 1.0\% 0\% | 1.4\% | - | 6.7\% | 1.4\% | 0\% 0\% |  | 1.4\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 1.3\% |
| Articulated Trucks | 03 | $0 \quad 0$ | 3 | - | 0 | 3 | 0 | 0 | 3 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 6 |
| \% Articulated Trucks | 0\% 0.3\% | 0\% 0\% | 0.2\% | - | 0\% | 0.3\% | 0\% 0\% |  | 0.3\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0.2\% |
| Buses | $0 \quad 12$ | 40 | 16 | - | 0 | 7 | 0 | 0 | 7 |  | 4 | 0 | 0 | 0 | 4 | - | 0 | 0 | 1 | 0 | 1 | - | 28 |
| \% Buses | 0\% 1.0\% | 4.0\% 0\% | 1.2\% | - | 0\% | 0.6\% | 0\% 0\% |  | 0.6\% |  | 14.3\% | 0\% | 0\% 0\% |  | 6.3\% |  | 0\% | 0\% | 3.0\% 0\% |  | 1.8\% |  | 1.1\% |
| Bicycles on Road | $0 \quad 0$ | $0 \quad 0$ | 0 | - | 0 | 1 | 0 | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | - | 1 |
| \% Bicycles on Road | 0\% 0\% | 0\% 0\% | 0\% |  | 0\% | 0.1\% | 0\% 0\% |  | 0.1\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% |
| Pedestrians | - - | - - | - | 1 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Pedestrians | - - | - - |  | 100\% | - | - | - | - | - |  | - | - | - | - | - |  | - - | - | - | - | - | - |  |
| Bicycles on Crosswalk | - - | - - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Bicycles on Crosswalk | - - | - - | - | 0\% | - | - |  |  | - |  | - - | - | - |  | - | - | - - | - | - | - | - | - | - |

[^0]Tue Mar 9, 2021
Full Length (7 AM-9 AM, 4 PM-6 PM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

ID: 817412, Location: 43.025598, -83.676457
[N] Walnut St
Total: 93
In: 57 Out: 36


Out: 118 In: 64
Total: 182
[S] Walnut St

Robert T Longway Boulevard \& Walnut Street - TMC
Tue Mar 9, 2021
AM Peak (7:30 AM - 8:30 AM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

ID: 817412, Location: 43.025598, -83.676457


[^1]Tue Mar 9, 2021
AM Peak (7:30 AM - 8:30 AM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements
ID: 817412, Location: 43.025598, -83.676457
[N] Walnut St
Total: 18
In: 11 Out: 7

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Out: $30 \quad$ In: 22
Total: 52
[S] Walnut St

Tue Mar 9, 2021
PM Peak (4 PM - 5 PM) - Overall Peak Hour
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

ID: 817412, Location: 43.025598, -83.676457

| Leg <br> Direction | Robert T Longway Blvd Eastbound |  |  |  |  |  | Robert T Longway Blvd Westbound |  |  |  |  |  | Walnut St Northbound |  |  |  |  |  | Walnut St <br> Southbound |  |  |  |  |  | Int |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R |  | App |  | L | T | R | U | App |  | L | T | R | U | App |  | L | T | R | U |  |  |  |
| 2021-03-09 4:00PM | 2 | 97 | 4 | 0 | 103 | 0 | 1 | 110 | 1 | 0 | 112 | 0 | 1 | 0 | 3 | 0 | 4 | 0 | 3 | 1 | 2 | 0 | 6 | 0 | 225 |
| 4:15PM | 2 | 99 | 2 | 0 | 103 | 0 | 1 | 71 | 0 | 0 | 72 | 0 | 1 | 2 | 2 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 182 |
| 4:30PM | 1 | 90 | 1 | 0 | 92 | 0 | 1 | 107 | 2 | 0 | 110 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 2 | 2 | 2 | 0 | 6 | 0 | 211 |
| 4:45PM | 0 | 102 | 5 | 0 | 107 | 0 | 1 | 81 | 1 | 0 | 83 | 0 | 3 | 0 | 1 | 0 | 4 | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 196 |
| Total | 5 | 388 | 12 | 0 | 405 | 0 | 4 | 369 | 4 | 0 | 377 | 0 | 8 | 2 | 6 | 0 | 16 | 0 | 6 | 3 | 7 | 0 | 16 | 0 | 814 |
| \% Approach | 1.2\% 9 | 95.8\% | 3.0\% 0\% |  | - |  | 1.1\% | 97.9\% | 1.1\% 0 |  | - |  | 50.0\% | 12.5\% | 37.5\% 0\% | \% | - |  | 37.5\% 18 | 18.8\% | 43.8\% 0\% |  | - |  |  |
| \% Total | 0.6\% | 47.7\% | 1.5\% 0\% | 0\% 4 | 49.8\% |  | 0.5\% | 45.3\% | 0.5\% 0 | 0\% | 46.3\% |  | 1.0\% | 0.2\% | 0.7\% 0\% |  | 2.0\% |  | 0.7\% | 0.4\% | 0.9\% 0\% | \% | 2.0\% |  |  |
| PHF | 0.625 | 0.951 | 0.600 |  | 0.946 |  | 1.000 | 0.8390 | 0.500 | - | 0.842 |  | 0.667 | 0.250 | 0.500 | - 0 | 0.800 |  | 0.500 | 0.375 | 0.875 |  | . 667 |  | 0.904 |
| Lights | 5 | 376 | 11 | 0 | 392 | - | 4 | 363 | 4 | 0 | 371 |  | 7 | 2 | 6 | 0 | 15 |  | 6 | 3 | 7 | 0 | 16 |  | 794 |
| \% Lights | 100\% 9 | 96.9\% | 91.7\% 0\% | 0\% 9 | 96.8\% |  | 100\% | 98.4\% 1 | 100\% 0 | 0\% | 98.4\% |  | 87.5\% | 100\% | 100\% 0\% | \% 9 | 93.8\% |  | 100\% | 100\% | 100\% 0\% | \% | 00\% |  | 97.5\% |
| Single-Unit Trucks | 0 | 5 | 0 | 0 | 5 | - | 0 | 5 | 0 | 0 | 5 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 10 |
| \% Single-Unit Trucks | 0\% | 1.3\% | 0\% 0\% | 0\% | 1.2\% |  | 0\% | 1.4\% | 0\% 0 |  | 1.3\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0\% | \% | 0\% |  | 1.2\% |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 |  | 0 | 1 | 0 | 0 | 1 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 |  | 1 |
| \% Articulated Trucks | 0\% | 0\% | 0\% 0\% | 0\% | 0\% |  | 0\% | 0.3\% | 0\% 0\% |  | 0.3\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0.1\% |
| Buses | 0 | 7 | 1 | 0 | 8 | - | 0 | 0 | 0 | 0 | 0 | - | - 1 | 0 | 0 | 0 | 1 | - | 0 | 0 | 0 | 0 | 0 |  | 9 |
| \% Buses | 0\% | 1.8\% | 8.3\% 0\% | \% | 2.0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% |  | 12.5\% | 0\% | 0\% 0\% | \% | 6.3\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 1.1\% |
| Bicycles on Road | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 |  | 0 |
| \% Bicycles on Road | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0 |  | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0\% |
| Pedestrians | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Pedestrians | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |  | - |
| Bicycles on Crosswalk | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Bicycles on Crosswalk | - | - | - | - | - | - | - - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |  | - |

[^2]PM Peak (4 PM - 5 PM) - Overall Peak Hour
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements
ID: 817412, Location: 43.025598, -83.676457

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US
[N] Walnut St
Total: 27
In: 16 Out: 11

Out: 19 In: 16
Total: 35
[S] Walnut St

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * $\uparrow$ |  |  | * $\uparrow$ |  |  | 4 |  |  | 4 |  |
| Traffic Vol, veh/h | 8 | 353 | 34 | 5 | 333 | 1 | 13 | 0 | 16 | 8 | 0 | 7 |
| Future Vol, veh/h | 8 | 353 | 34 | 5 | 333 | 1 | 13 | 0 | 16 | 8 | 0 | 7 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 86 | 86 | 86 | 60 | 60 | 60 | 69 | 69 | 69 |
| Heavy Vehicles, \% | 3 | 3 | 3 | 4 | 4 | 4 | 5 | 5 | 5 | 0 | 0 | 0 |
| Mvmt Flow | 8 | 372 | 36 | 6 | 387 | 1 | 22 | 0 | 27 | 12 | 0 | 10 |



Intersection: 1: Walnut St \& Robert T Longway Blvd

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | LT | LTR | LTR |
| Maximum Queue (ft) | 36 | 32 | 57 | 41 |
| Average Queue (ft) | 3 | 2 | 20 | 12 |
| 95th Queue (ft) | 17 | 14 | 48 | 38 |
| Link Distance (ft) | 457 | 422 | 261 | 316 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
|  |  |  |  |  |
| Network Summary |  |  |  |  |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * $\uparrow$ |  |  | * $\uparrow$ |  |  | \& |  |  | 4 |  |
| Traffic Vol, veh/h | 7 | 505 | 16 | 5 | 480 | 5 | 10 | 3 | 8 | 8 | 4 | 9 |
| Future Vol, veh/h | 7 | 505 | 16 | 5 | 480 | 5 | 10 | 3 | 8 | 8 | 4 | 9 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 84 | 84 | 84 | 80 | 80 | 80 | 67 | 67 | 67 |
| Heavy Vehicles, \% | 3 | 3 | 3 | 2 | 2 | 2 | 6 | 6 | 6 | 0 | 0 | 0 |
| Mvmt Flow | 7 | 532 | 17 | 6 | 571 | 6 | 13 | 4 | 10 | 12 | 6 | 13 |



Intersection: 1: Walnut St \& Robert T Longway Blvd

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | LT | LTR | LTR |
| Maximum Queue (ft) | 57 | 36 | 49 | 48 |
| Average Queue (ft) | 5 | 2 | 15 | 17 |
| 95th Queue (ft) | 32 | 16 | 41 | 44 |
| Link Distance (ft) | 452 | 413 | 250 | 301 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
|  |  |  |  |  |
| Network Summary |  |  |  |  |

## Robert T Longway Blvd and Walnut Street - 2 by 1 - AM Peak



## Robert T Longway Blvd and Walnut Street - 2 by 1 - PM Peak

T. Rodel - C:\Users\902JAM 1 OneDrive - ROWE PSC\19C0262_GeneseeCountyRAB\RODEL\RobertTLongway_Walnut.rod

File View Help


## Robert T Longway Blvd and Walnut Street - Single - AM Peak

T. Rodel - C:\Users\902JAM\OneDrive - ROWE PSC\19C0262_GeneseeCountyRAB\RODEL\RobertTLongway_Walnut_Single.rod

File View Help


## Robert T Longway Blvd and Walnut Street - Single - PM Peak

- Rodel - C:\Users\902JAM MOneDrive - ROWE PSC\19C0262_GeneseeCountyRAB\RODEL\RobertTLongway_Walnut_Single.rod

File View Help

T. Rodel - R:\Projects\19C0262\Docs\Design\RobertTLongway_Walnut_Whiting\RobertTLongway_Walnut_Single_Whiting_LOS.rod


Genesee County Roundabout Study - EPE Analysis

| Intersection |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Robert T Longway Blvd and Walnut Street |  |  |  |  |
| Opinion of Probable Cost |  |  |  |  |
| By: ROWE <br> Date: 7/9/2021 |  |  |  |  |
|  |  |  |  |  |
| PAY ITEM DESCRIPTION | ESTIMATED QUANTITY | UNIT | UNIT PRICE | AMOUNT |
| Mobilization (10\%) 1 LSUM $\$ 76,000.00$ $\$ 76,000.00$ |  |  |  |  |
| S\|c|r|r|r| |  |  |  |  |
| Pavt, Rem $\quad 4600$ Sy |  |  |  |  |
| Curb and Gutter, Rem 2000 Ft $\$ 10.00$ $\$ 20,000.00$ |  |  |  |  |
| Embankment, CIP |  |  |  |  |
|  |  |  |  |  |
| Aggregate Base | 2200 | Ton | \$21.00 | \$46,200.00 |
|  |  |  |  |  |
| HMA Approach 15 Ton $\$ 50.00$ |  |  |  |  |
| Conc Pavt, Nonreinf, 9 inch 2610 Syd $\$ 45.00$ $\$ 117,450.00$ |  |  |  |  |
| Joint, Contraction, Cp 2400 Ft $\$ 10.00$ $\$ 24,000.00$ |  |  |  |  |
| Joint, Expansion, E3 1300 Ft \$15.00 $\$ 19,500.00$ |  |  |  |  |
| Driveway, Nonreinf Conc, 9 inch 150 Syd $\$ 50.00$ $\$ 7,500.00$ |  |  |  |  |
| Curb and Gutter, Conc, Det B1 2500 Ft $\$ 25.00$ $\$ 62,500.00$ |  |  |  |  |
| Curb and Gutter, Conc, Det D1 230 Ft $\$ 25.00$ $\$ 5,750.00$ |  |  |  |  |
| Curb, Conc. Det E1 110 Ft $\$ 25.00$ $\$ 2,750.00$ |  |  |  |  |
| Driveway Opening, Conc, Det M 140 Ft $\$ 22.00$ $\$ 3,080.00$ |  |  |  |  |
|  |  |  |  |  |
| Curb Ramp Opening, Conc 14 Ft $\$ 25.00$ |  |  |  |  |
|  |  |  |  |  |
| Sidewalk Ramp, Conc, 6 inch 100 Sft $\$ 10.00$ $\$ 1,000.00$ |  |  |  |  |
| Conc Pavt, Decorative Colored, 9 inch 5200 Sft $\$ 12.50$ $\$ 65,000.00$ |  |  |  |  |
| Turf Establishment, Performance 5000 Syd $\$ 5.00$ $\$ 25,000.00$ |  |  |  |  |
|  |  |  |  |  |
| Drainage 1 LSUM $\$ 100,000.00$ $\$ 100,000.00$ |  |  |  |  |
| Pavement Markings 1 LSUM $\$ 15,000.00$ $\$ 15,000.00$ |  |  |  |  |
| Signing 1 LSUM $\$ 20,000.00$ $\$ 20,000.00$ |  |  |  |  |
| Relocation of 46kV Consumers Energy Poles 1 LSUM $\$ 200,000.00$ $\$ 200,000.00$ |  |  |  |  |
|  |  |  |  |  |
| TOTAL $\quad$ \$1,032,580 |  |  |  |  |
|  |  | CON | VGENCY (20\%) | \$206,516.00 |
|  | ESTIMATED TOTAL CONSTR | CTION C | ( Y ( AR 2021) | \$1,239,096.00 |
|  | 3\% ANNUAL | NFLATIO | 2021 TO 2025 | \$155,517.46 |
|  | ESTIMATED TOTAL CONSTR | CTION COS | T (YEAR 2025) | \$1,394,613.46 |

## Assumptions:

6" Agg base used for driveways and shoulders, 8 " used for roadway, 12 " used for islands ( $10 \%$ of total added)
Asphalt shoulder gravel thickness was calculated using a 5" depth
Assumed 5" HMA thickness for approach and 8" thick for HMA roadway

B1 curb was calculated by adding the splitter islands along with the roadways, minus the radius for the roundabout E1 curb was calculated for the inner roundabout curb
E2 joint expansion was caclulated for the outer radius of the roundabout
E3 joint expansion was caclulated using the radius of curves from the $B / C$ of the roundabout and splitter islands D1 curb was calculated for the truck apron
Drainage includes enclosed storm for all curbed areas, spillways, and underdrain
MOT taken as approximately $\sim 8 \%$ of total before mobilization; based on previous roundabout projects
ROW acqusition costs not inlcuded
Joints assumed for central island/splitter islands

Genesee County Roundabout Study - EPE Analysis


## Assumptions:

6" Agg base used for driveways and shoulders, 8" used for roadway, 12 " used for islands ( $10 \%$ of total added)
Asphalt shoulder gravel thickness was calculated using a 5 " depth
Assumed 5" HMA thickness for approach and 8" thick for HMA roadway
B1 curb was calculated by adding the splitter islands along with the roadways, minus the radius for the roundabout E1 curb was calculated for the inner roundabout curb
E2 joint expansion was caclulated for the outer radius of the roundabout
E3 joint expansion was caclulated using the radius of curves from the $B / C$ of the roundabout and splitter islands
D1 curb was calculated for the truck apron
Drainage includes enclosed storm for all curbed areas, spillways, and underdrain
MOT taken as approximately $\sim 8 \%$ of total before mobilization; based on previous roundabout projects
ROW acqusition costs not inlcuded
Joints assumed for central island/splitter islands

NUMBER OF CRASHES OR INJURED PERSONS.


REMARKS:

```
Robert T Longway Blvd and Walnut Street
Genesee County Roundabout Study
1497401,1535107
0.4819510, 0.1104172
Roundabout
```


# COMPUTED BENEFITS DERIVED THROUGH CRASH REDUCTION 

## TOR 2021

Project: Robert T Longway Blvd and Walnut Street
Prepared By: ROWE Professional Services Company

City/Twp. City of Flint County Genesee County

PR: 1497401, $1535107 \quad$ PR MP Range: $\mathbf{0 . 4 8 1 9 5 1 0 , 0 . 1 1 0 4 1 7 2 ~}$

The method of evaluating crash costs, used below, is given on page 67 of Roy Jorgensen's report of Highway Safety Improvement Criteria, 1966 edition. This same method is given in the Bureau of Public Roads IM21-3-67. In 1994 we have adapted the $Q$ formula to blend Fatalities and $A$-injuries only. In the following analysis the costs provided by the National Safety Council are :

2019 NSC VALUES:

| Death | $\$ 1,659,000$ | $=$ FATCOST |
| :--- | ---: | :--- |
| Disabling (A) injury: | $\$ 96,200$ | $=$ ACOST |
| B-injury: | $\$ 27,800=$ BCOST |  |
| PDO and/or Minor Injury Crash: | $\$ 12,200$ | $=$ PDOCOST |
|  |  |  |
| BTOTAL $=$ ADTa $/$ ADTb $\times[(Q \times R 1)+(B C O S T \times R 2)+(P D O C O S T \times R 3)]$ |  |  |

WHERE:

BTOTAL $=$ Total Benefit in Dollars Over Years Used $\quad \$ 53,546$
ADTa $=\quad$ Average traffic volume after the improvement 1.1
ADTb $=\quad$ Average traffic volume before the improvement 1.0
R1 $=$ Reduction in fatalities and A-Injuries Combined. 0.0
R2 $=$ Reduction in B-Injury crashes: 0.0
R3 $=$ Reduction in PDO and C-injury crashes: 4.0
$\mathrm{Q}=[$ FATCOST $+((\mathrm{I} / \mathrm{F}) \times$ INJCOST $)] /[1+(\mathrm{I} / \mathrm{F})]$
$=\quad[1,659,000+(6.10 \times 96,200)] /[1+6.10]$
\$316,400
for AREA TYPE "Urban"
I/F =
6.10

| Q-Reference | Q | A-Inuries | Fatalities | I/F |
| :--- | ---: | ---: | ---: | ---: |
| RURAL | $\$ 363,900$ | 6,072 | 1,255 | 4.84 |
| URBAN | $\$ 316,400$ | 9,902 | 1,624 | 6.10 |
| BETWEEN | $\$ 334,900$ | 15,974 | 2,879 | 5.55 |

Data from Safety Programs Unit
5-Year Statewide Non-Trunkline Crash Figures Used.
(*From 1-1-2015 Through 12-31-2019).

Time of Return (T.O.R.) is based on ....

NOINFB $=$ No-Inflation Annual Benefit=BTOTAL/years

With an inflation rate of $\qquad$
$B=A n n u a l$ Benefit=Present Value (with Inflation)

C = Project Cost

TOR=C/B=COST/ANNUAL BENEFIT=
\$10,709
2.50\%
\$13,709
\$1,394,613
101.73
L. Robert T. Longway and Walnut

1. 2045 AM Peak Hour No Build
2. 2045 PM Peak Hour No Build
3. 2045 AM Peak Hour Single Roundabout
4. 2045 PM Peak Hour Single Roundabout
5. 2045 AM Peak Hour Two-by-One Roundabout
6. 2045 PM Peak Hour Two-by-One Roundabout

[^0]:    *Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

[^1]:    *Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

[^2]:    *Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

