## 2015-2019 CRASH DATA



Opinion of probable cost for single-lane roundabout
\$1.25 MILLION

## H. Lapeer Road and S. Genesee Road

The intersection of Lapeer Road and S. Genesee Road was included in the early preliminary engineering phase with support from the City of Burton due to intersection operations and safety concerns. This intersection is included in the 153 intersections that passed the skim analysis for experiencing crashes correctable by a roundabout and is a secondary analysis Tier Two intersection.

During the skim analysis, it was observed that there were 13 angle crashes and two head on left turn crashes over the 5-year period. These crashes resulted in zero fatalities, one type A injury, two type $B$ injuries, and six type $C$ injuries at the intersection.

## No-Build Conditions

The intersection of Lapeer Road and S. Genesee Road is a four-way stop controlled intersection. Lapeer Road is an east/west road with two lanes in each direction. S. Genesee Road is a north/south road also with two lanes in each direction. All approaches of the intersection widen out from a singlelane to have a right-turn only lane and a shared through/left-turn lane. There are overhead flashing traffic signals (red) for traffic control for all approaches.

In the southeast quadrant of the intersection is the Lapeer Market. The northwest and southwest quadrants present an area of fields near residential homes. The northeast quadrant presents a residential home and a sidewalk going north and east. No other quadrants at the intersections have sidewalks. There are overhead utilities crossing the south and east legs. For utilities present at ground level, there is a fire hydrant in the northwest quadrant, drainage structures in the northeast, northwest, and southeast quadrants, and gas line markers in the southeast and south west quadrant.

An aerial of the existing intersection can be seen in Figure 9.

Figure 9: Aerial view of Lapeer Road and S. Genesee Road


An operational analysis of the no-build condition was completed for the intersection using the 2045 forecast traffic volumes. The results of the analysis for the future no-build condition reveals all approaches and movements of the intersection operate at LOS B during the AM peak hour. During the PM peak hour, the east and westbound approaches operate at LOS C and northbound operates at LOS D and southbound at LOS E.

The $95^{\text {th }}$ percentile queue lengths were reviewed at the intersection and the results showed all approaches experienced a maximum queue length of 84 feet ( 5 vehicles) during the AM peak hour and 152 feet ( 10 vehicles) during the PM peak hour.

The operational analysis results for the future no-build conditions are presented in Table 20.

Table 20: Operational Analysis for 2045 No-Build Condition

| Intersection |  | AM Peak |  | PM Peak |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Delay/LOS | Queue (veh)* | Delay/LOS | Queue (veh)* |
| Lapeer Road and S. Genesee Road | Eastbound | $13.4 / \mathrm{B}$ | $3(55 \mathrm{ft})$ | $20.2 / \mathrm{C}$ | $5(83 \mathrm{ft})$ |
|  | Westbound | $12.1 / \mathrm{B}$ | $4(60 \mathrm{ft})$ | $17.4 / \mathrm{C}$ | $4(71 \mathrm{ft})$ |
|  | Northbound | $14.1 / \mathrm{B}$ | $4(65 \mathrm{ft})$ | $30.9 / \mathrm{D}$ | $8(127 \mathrm{ft})$ |
|  | Southbound | $14.9 / \mathrm{B}$ | $5(84 \mathrm{ft})$ | $41.9 / \mathrm{E}$ | $10(152 \mathrm{ft})$ |
|  | Overall | $13.8 / \mathrm{B}$ |  | $29.9 / \mathrm{D}$ |  |

[^0]
## Roundabout Conditions

The proposed roundabout configuration for the intersection of Lapeer Road and S. Genesee Road is a single lane circulating. The proposed single-lane roundabout configuration will not fit inside the existing right-of-way. Based on the concept design, additional right-of-way may be required in the northeast, southeast, and southwest. The proposed inscribed diameter for the concept roundabout is 110 feet. The driveway configuration for the Lapeer Road Market should be investigated due to the existing driveways closeness to the proposed roundabout entry/exit radius. A concept plan for the single-lane roundabout is to follow the recommendations.

An operational analysis for the single-lane roundabout (build) condition was completed for the intersection using 2045 forecast traffic volumes. The results of the analysis for the roundabout (build) condition reveals all approaches and movements of the intersection operate at LOS A during the AM and PM peak hours.

The $95^{\text {th }}$ percentile queue lengths were reviewed at the intersection and the results showed all approaches experienced a maximum queue length of one (1) vehicle during the AM peak hour and two (2) vehicles during the PM peak hour.

The operation analysis for the future roundabout (build) conditions are presented in Table 21.

Table 21: Operational Analysis for 2045 Roundabout (Build) Condition

| Intersection | Approach | AM Peak |  | PM Peak |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Delay/LOS | Queue (veh)* | Delay/LOS | Queue (veh)* |
| Lapeer Road and S. Genesee Road | Eastbound | $5.2 / \mathrm{A}$ | 0.5 | $7.0 / \mathrm{A}$ | 1.0 |
|  | Westbound | $6.1 / \mathrm{A}$ | 0.8 | $6.7 / \mathrm{A}$ | 0.9 |
|  | Northbound | $5.6 / \mathrm{A}$ | 0.7 | $7.7 / \mathrm{A}$ | 1.5 |
|  | Southbound | $6.9 / \mathrm{A}$ | 1.2 | $8.2 / \mathrm{A}$ | 2.1 |
|  | Overall | $6.1 / \mathrm{A}$ |  | 7.6/A |  |

* $95^{\text {th }}$ percentile queue length.

Opinion of probable cost were developed for a single-lane roundabout. The total probable cost is $\$ 1.25$ million in year 2025 dollars. The probable cost includes a 20 percent contingency and 3 percent inflation. Not included in this fee are the potential costs to relocate any utilities to accommodate the proposed layout. A full breakdown along with all the assumptions can be found in Appendix 3.

Potential funding sources for this improvement could include regular road improvement funding, safety funds, CMAQ, or an earmark. A TOR was computed with a result of 10.66.

## Recommendation

A roundabout would be feasible at this location, although it may be costly due to the necessary relocation of the utilities. There also should be coordination with the Lapeer Road Market about their driveway configuration.


Tue Mar 9, 2021
Full Length (7 AM-9 AM, 4 PM-6 PM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

ID: 817409, Location: 43.010534, -83.615019

| Leg <br> Direction | Lapeer Rd Eastbound |  |  |  |  |  | Lapeer Rd Westbound |  |  |  |  |  | S Genesee Rd Northbound |  |  |  |  |  | S Genesee Rd Southbound |  |  |  |  |  | Int |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R | U | App | Ped* | L | T | R | U | App | Ped* | L | T | R | U | App | ed* | L | T | R | U |  |  |  |
| 2021-03-09 7:00AM | 3 | 8 | 1 | 0 | 12 | 0 | 2 | 12 | 7 | 0 | 21 | 0 | 5 | 13 | 1 | 0 | 19 | 0 | 4 | 12 | 10 | 0 | 26 | 0 | 78 |
| 7:15AM | 7 | 10 | 5 | 0 | 22 | 0 | 2 | 23 | 1 | 0 | 26 | 0 | 3 | 21 | 1 | 0 | 25 | 0 | 3 | 8 | 2 | 0 | 13 | 0 | 86 |
| 7:30AM | 13 | 16 | 3 | 0 | 32 | 0 | 5 | 21 | 16 | 0 | 42 | 0 | 7 | 19 | 3 | 0 | 29 | 0 | 6 | 18 | 4 | 0 | 28 | 0 | 131 |
| 7:45AM | 5 | 17 | 4 | 0 | 26 | 0 | 5 | 24 | 17 | 0 | 46 | 0 | 6 | 30 | 3 | 0 | 39 | 0 | 5 | 37 | 18 | 0 | 60 | 0 | 171 |
| Hourly Total | 28 | 51 | 13 | 0 | 92 | 0 | 14 | 80 | 41 | 0 | 135 | 0 | 21 | 83 | 8 | 0 | 112 | 0 | 18 | 75 | 34 | 0 | 127 | 0 | 466 |
| 8:00AM | 25 | 14 | 1 | 0 | 40 | 1 | 2 | 29 | 16 | 0 | 47 | 0 | 10 | 22 | 3 | 0 | 35 | 0 | 14 | 34 | 33 | 0 | 81 | 0 | 203 |
| 8:15AM | 3 | 16 | 5 | 0 | 24 | 0 | 8 | 17 | 10 | 0 | 35 | 0 | 7 | 22 | 4 | 0 | 33 | 0 | 8 | 32 | 14 | 0 | 54 | 0 | 146 |
| 8:30AM | 5 | 10 | 5 | 1 | 21 | 0 | 4 | 20 | 15 | 0 | 39 | 0 | 7 | 37 | 3 | 0 | 47 | 0 | 8 | 29 | 6 | 0 | 43 | 0 | 150 |
| 8:45AM | 8 | 11 | 2 | 0 | 21 | 0 | 7 | 10 | 13 | 0 | 30 | 0 | 5 | 24 | 3 | 0 | 32 | 0 | 7 | 38 | 5 | 0 | 50 | 0 | 133 |
| Hourly Total | 41 | 51 | 13 | 1 | 106 | 1 | 21 | 76 | 54 | 0 | 151 | 0 | 29 | 105 | 13 | 0 | 147 | 0 | 37 | 133 | 58 | 0 | 228 | 0 | 632 |
| 4:00PM | 8 | 26 | 5 | 0 | 39 | 0 | 6 | 36 | 12 | 0 | 54 | 0 | 5 | 65 | 4 | 0 | 74 | 0 | 19 | 57 | 16 | 0 | 92 | 0 | 259 |
| 4:15PM | 16 | 26 | 9 | 0 | 51 | 0 | 6 | 27 | 11 | 0 | 44 | 0 | 4 | 47 | 6 | 0 | 57 | 0 | 20 | 48 | 11 | 0 | 79 | 0 | 231 |
| 4:30PM | 10 | 32 | 7 | 0 | 49 | 0 | 5 | 23 | 18 | 0 | 46 | 0 | 5 | 45 | 5 | 0 | 55 | 0 | 22 | 49 | 6 | 0 | 77 | 0 | 227 |
| 4:45PM | 14 | 33 | 5 | 0 | 52 | 0 | 9 | 20 | 7 | 0 | 36 | 0 | 6 | 58 | 11 | 0 | 75 | 0 | 21 | 52 | 18 | 0 | 91 | 0 | 254 |
| Hourly Total | 48 | 117 | 26 | 0 | 191 | 0 | 26 | 106 | 48 | 0 | 180 | 0 | 20 | 215 | 26 | 0 | 261 | 0 | 82 | 206 | 51 | 0 | 339 | 0 | 971 |
| 5:00PM | 19 | 37 | 8 | 0 | 64 | 0 | 6 | 22 | 10 | 0 | 38 | 0 | 12 | 46 | 7 | 0 | 65 | 0 | 13 | 56 | 12 | 0 | 81 | 0 | 248 |
| 5:15PM | 10 | 45 | 10 | 0 | 65 | 0 | 5 | 24 | 14 | 0 | 43 | 0 | 6 | 49 | 1 | 0 | 56 | 0 | 22 | 38 | 4 | 0 | 64 | 0 | 228 |
| 5:30PM | 16 | 30 | 9 | 0 | 55 | 0 | 4 | 20 | 14 | 0 | 38 | 0 | 4 | 42 | 4 | 0 | 50 | 0 | 17 | 24 | 7 | 0 | 48 | 0 | 191 |
| 5:45PM | 13 | 27 | 5 | 0 | 45 | 0 | 12 | 30 | 10 | 0 | 52 | 1 | 3 | 43 | 2 | 0 | 48 | 0 | 11 | 44 | 14 | 0 | 69 | 0 | 214 |
| Hourly Total | 58 | 139 | 32 | 0 | 229 | 0 | 27 | 96 | 48 | 0 | 171 | 1 | 25 | 180 | 14 | 0 | 219 | 0 | 63 | 162 | 37 | 0 | 262 | 0 | 881 |
| Total | 175 | 358 | 84 | 1 | 618 | 1 | 88 | 358 | 191 | 0 | 637 | 1 | 95 | 583 | 61 | 0 | 739 | 0 | 200 | 576 | 180 | 0 | 956 | 0 | 2950 |
| \% Approach | 28.3\% | 57.9\% | 13.6\% | 0.2\% | - |  | 13.8\% 5 | 56.2\% | 30.0\% 0 |  | - |  | 12.9\% 7 | 78.9\% | 8.3\% 0\% |  | - |  | 20.9\% | 60.3\% | 18.8\% |  | - |  | - |
| \% Total | 5.9\% | 12.1\% | 2.8\% | 0\% | 20.9\% |  | 3.0\% 1 | 12.1\% | 6.5\% 0 | 0\% | 21.6\% |  | 3.2\% | 19.8\% | 2.1\% 0\% | \% | 25.1\% |  | 6.8\% | 19.5\% | 6.1\% | \% | 3.4\% |  |  |
| Lights | 171 | 355 | 81 | 1 | 608 |  | 88 | 352 | 185 | 0 | 625 |  | 91 | 575 | 60 | 0 | 726 |  | 195 | 572 | 172 | 0 | 939 |  | 2898 |
| \% Lights | 97.7\% | 99.2\% 9 | 96.4\% | 100\% | 98.4\% |  | 100\% 9 | 98.3\% | 96.9\% 0 | 0\% | 98.1\% |  | 95.8\% | 98.6\% | 98.4\% 0\% | \% 9 | 98.2\% |  | 97.5\% | 99.3\% | 95.6\% | \% | 8.2\% |  | 98.2\% |
| Single-Unit Trucks | 0 | 2 | 0 | 0 | 2 | - | 0 | 5 | 2 | 0 | 7 | - | 1 | 6 | 0 | 0 | 7 |  | 3 | 0 | 0 | 0 | 3 |  | 19 |
| \% Single-Unit Trucks | 0\% | 0.6\% | 0\% | 0\% | 0.3\% |  | 0\% | 1.4\% | 1.0\% 0 |  | 1.1\% |  | 1.1\% | 1.0\% | 0\% 0\% | 0\% | 0.9\% |  | 1.5\% | 0\% | 0\% | \%\% | 0.3\% |  | 0.6\% |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 |  | 0 | 1 | 1 | 0 | 2 | - | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 2 |
| \% Articulated Trucks | 0\% | 0\% | 0\% | 0\% | 0\% | - | 0\% | 0.3\% | 0.5\% 0 | 0\% | 0.3\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% |  | 0\% |  | 0.1\% |
| Buses | 4 | 1 | 3 | 0 | 8 |  | 0 | 0 | 1 | 0 | 1 |  | 2 | 2 | 1 | 0 | 5 |  | 2 | 4 | 8 | 0 | 14 |  | 28 |
| \% Buses | 2.3\% | 0.3\% | 3.6\% | 0\% | 1.3\% | - | 0\% | 0\% | 0.5\% 0 | 0\% | 0.2\% | - | 2.1\% | 0.3\% | 1.6\% 0\% | 0\% | 0.7\% |  | 1.0\% | 0.7\% | 4.4\% | 0\% | 1.5\% |  | 0.9\% |
| Bicycles on Road | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 2 | 0 | 2 | - | 1 | 0 | 0 | 0 | 1 | - | 0 | 0 | 0 | 0 | 0 |  | 3 |
| \% Bicycles on Road | 0\% | 0\% | 0\% | 0\% | 0\% |  | 0\% | 0\% | 1.0\% 0 |  | 0.3\% |  | 1.1\% | 0\% | 0\% 0\% |  | 0.1\% |  | 0\% | 0\% | 0\% |  | 0\% |  | 0.1\% |
| Pedestrians | - | - | - | - | - | 1 | - | - | - - | - | - | 1 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Pedestrians | - | - | - | - |  | 100\% | - |  | - | - |  | 100\% | - | - | - | - | - |  | - | - | - | - | - | - | - |
| Bicycles on Crosswalk | - | - | - | - | - | 0 | - |  | - |  | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Bicycles on Crosswalk | - | - | - | - | - | 0\% | - | - | - | - | - | 0\% | - | - | - | - | - | - | - | - | - | - | - | - | - |

*Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn
[N] S Genesee Rd
Total: 1905
In: 956
Out: 949


Tue Mar 9, 2021
AM Peak (7:45 AM - 8:45 AM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

Provided by: Gewalt Hamilton Associates Inc.
All Movements
ID: 817409, Location: 43.010534, -83.615019

| Leg <br> Direction | Lapeer Rd <br> Eastbound |  |  |  |  |  | Lapeer Rd Westbound |  |  |  |  |  | S Genesee Rd Northbound |  |  |  |  |  | S Genesee Rd Southbound |  |  |  |  |  | Int |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R | U | App | Ped* | L | T | R |  | App |  | L | T | R | U | App |  | L | T | R | U |  |  |  |
| 2021-03-09 7:45AM | 5 | 17 | 4 | 0 | 26 | 0 | 5 | 24 | 17 | 0 | 46 | 0 | 6 | 30 | 3 | 0 | 39 | 0 | 5 | 37 | 18 | 0 | 60 | 0 | 171 |
| 8:00AM | 25 | 14 | 1 | 0 | 40 | 1 | 2 | 29 | 16 | 0 | 47 | 0 | 10 | 22 | 3 | 0 | 35 | 0 | 14 | 34 | 33 | 0 | 81 | 0 | 203 |
| 8:15AM | 3 | 16 | 5 | 0 | 24 | 0 | 8 | 17 | 10 | 0 | 35 | 0 | 7 | 22 | 4 | 0 | 33 | 0 | 8 | 32 | 14 | 0 | 54 | 0 | 146 |
| 8:30AM | 5 | 10 | 5 | 1 | 21 | 0 | 4 | 20 | 15 | 0 | 39 | 0 | 7 | 37 | 3 | 0 | 47 | 0 | 8 | 29 | 6 | 0 | 43 | 0 | 150 |
| Total | 38 | 57 | 15 | 1 | 111 | 1 | 19 | 90 | 58 | 0 | 167 | 0 | 30 | 111 | 13 | 0 | 154 | 0 | 35 | 132 | 71 | 0 | 238 | 0 | 670 |
| \% Approach | 34.2\% 5 | 51.4\% 1 | 13.5\% | 0.9\% | - |  | 11.4\% 5 | 53.9\% | 34.7\% 0\% |  | - |  | 19.5\% 7 | 72.1\% | 8.4\% 0\% | \% | - |  | 14.7\% | 55.5\% | 29.8\% 0 |  |  |  | - |
| \% Total | 5.7\% | 8.5\% | 2.2\% | 0.1\% | 16.6\% |  | 2.8\% | 13.4\% | 8.7\% 0\% | \% 2 | 24.9\% |  | 4.5\% 1 | 16.6\% | 1.9\% 0\% | \% | 23.0\% |  | 5.2\% | 19.7\% | 10.6\% 0 | \% | 35.5\% |  |  |
| PHF | 0.380 | 0.838 | 0.750 | 0.250 | 0.694 |  | 0.594 | 0.776 | 0.853 | - 0 | 0.888 |  | 0.750 | 0.750 | 0.813 | - | 0.819 |  | 0.625 | 0.892 | 0.538 |  | 0.735 |  | 0.825 |
| Lights | 37 | 56 | 14 | 1 | 108 | - | 19 | 89 | 56 | 0 | 164 | - | 30 | 110 | 13 | 0 | 153 |  | 33 | 129 | 68 | 0 | 230 |  | 655 |
| \% Lights | 97.4\% | 98.2\% | 93.3\% 1 | 100\% | 97.3\% |  | 100\% | 98.9\% | 96.6\% 0\% | \% 9 | 98.2\% |  | 100\% 9 | 99.1\% | 100\% 0\% | \% 9 | 99.4\% |  | 94.3\% | 97.7\% | 95.8\% 0 | \% 9 | 6.6\% |  | 97.8\% |
| Single-Unit Trucks | 0 | 1 | 0 | 0 | 1 |  | 0 | 1 | 0 | 0 | 1 |  | 0 | 1 | 0 | 0 | 1 |  | 1 | 0 | 0 | 0 | 1 |  | 4 |
| \% Single-Unit Trucks | 0\% | 1.8\% | 0\% | 0\% | 0.9\% |  | 0\% | 1.1\% | 0\% 0\% | \% | 0.6\% |  | 0\% | 0.9\% | 0\% 0\% | \% | 0.6\% |  | 2.9\% | 0\% | 0\% 0\% | \% | 0.4\% |  | 0.6\% |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 1 | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 1 |
| \% Articulated Trucks | 0\% | 0\% | 0\% | 0\% | 0\% | - | 0\% | 0\% | 1.7\% 0\% | \% | 0.6\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0.1\% |
| Buses | 1 | 0 | 1 | 0 | 2 |  | 0 | 0 | 1 | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 1 | 3 | 3 | 0 | 7 |  | 10 |
| \% Buses | 2.6\% | 0\% | 6.7\% | 0\% | 1.8\% | - | 0\% | 0\% | 1.7\% 0\% | \% | 0.6\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 2.9\% | 2.3\% | 4.2\% 0 |  | 2.9\% |  | 1.5\% |
| Bicycles on Road | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 |
| \% Bicycles on Road | 0\% | 0\% | 0\% | 0\% | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0\% |
| Pedestrians | - | - | - | - | - | 1 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Pedestrians | - | - | - | - |  | 100\% | - | - | - | - | - |  | - | - | - | - | - |  | - | - | - | - | - |  | - |
| Bicycles on Crosswalk | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Bicycles on Crosswalk | - | - | - | - | - | 0\% | - | - | - | - | - |  | - | - | - | - | - |  | - | - | - | - | - | - | $-$ |

*Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

AM Peak (7:45 AM - 8:45 AM)
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)
All Movements
ID: 817409, Location: 43.010534, -83.615019

Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US
[N] S Genesee Rd
Total: 445
In: 238
Out: 207


Out: 166 In: 154
Total: 320
[S] S Genesee Rd

Tue Mar 9, 2021
PM Peak (4 PM - 5 PM) - Overall Peak Hour
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

All Movements
ID: 817409, Location: 43.010534, -83.615019

| Leg <br> Direction | Lapeer Rd <br> Eastbound |  |  |  |  |  | Lapeer Rd Westbound |  |  |  |  |  | S Genesee Rd Northbound |  |  |  |  |  | S Genesee Rd Southbound |  |  |  |  |  | Int |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R |  | App |  | L | T | R |  | App |  | L | T | R | U | App |  | L | T | R | U | App |  |  |
| 2021-03-09 4:00PM | 8 | 26 | 5 | 0 | 39 | 0 | 6 | 36 | -12 | 0 | 54 | 0 | 5 | 65 | 4 | 0 | 74 | 0 | 19 | 57 | 16 | 0 | 92 | 0 | 259 |
| 4:15PM | 16 | 26 | 9 | 0 | 51 | 0 | 6 | 27 |  | 0 | 44 | 0 | 4 | 47 | 6 | 0 | 57 | 0 | 20 | 48 | 11 | 0 | 79 | 0 | 231 |
| 4:30PM | 10 | 32 | 7 | 0 | 49 | 0 | 5 | 23 | 18 | 0 | 46 | 0 | 5 | 45 | 5 | 0 | 55 | 0 | 22 | 49 | 6 | 0 | 77 | 0 | 227 |
| 4:45PM | 14 | 33 | 5 | 0 | 52 | 0 | 9 | 20 | 7 | 0 | 36 | 0 | 6 | 58 | 11 | 0 | 75 | 0 | 21 | 52 | 18 | 0 | 91 | 0 | 254 |
| Total | 48 | 117 | 26 | 0 | 191 | 0 | 26 | 106 | 48 | 0 | 180 | 0 | 20 | 215 | 26 | 0 | 261 | 0 | 82 | 206 | 51 | 0 | 339 | 0 | 971 |
| \% Approach | 25.1\% | 61.3\% | 13.6\% 0\% |  | - | - | 14.4\% | 58.9\% | 26.7\% 0 |  | - | - | 7.7\% | 82.4\% | 10.0\% 0 | \% | - |  | 24.2\% | 60.8\% | 15.0\% 0 |  | - |  | - |
| \% Total | 4.9\% | 12.0\% | 2.7\% 0\% | \% 19 | 9.7\% |  | 2.7\% | 10.9\% | 4.9\% 0 | 0\% 18 | 8.5\% | - | 2.1\% | 22.1\% | 2.7\% 0\% | \% | 26.9\% |  | 8.4\% | 21.2\% | 5.3\% 0\% | \% | 34.9\% |  |  |
| PHF | 0.750 | 0.886 | 0.722 |  | 0.918 | - | 0.722 | 0.736 | 0.667 | - 0 | 0.833 |  | 0.833 | 0.827 | 0.591 | - | 0.870 |  | 0.932 | 0.904 | 0.708 |  | 0.921 |  | 0.937 |
| Lights | 47 | 117 | 26 | 0 | 190 | - | 26 | 104 | 48 | 0 | 178 |  | 20 | 212 | 26 | 0 | 258 |  | 81 | 206 | 50 | 0 | 337 |  | 963 |
| \% Lights | 97.9\% | 100\% | 100\% 0\% | \% 99 | 99.5\% | - | 100\% | 98.1\% | 100\% 0\% | 0\% 98 | 98.9\% | - | 100\% | 98.6\% | 100\% 0\% | \% | 98.9\% |  | 98.8\% | 100\% | 98.0\% 0\% | \% 9 | 9.4\% |  | 99.2\% |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | 0 | - | 0 | 2 | 0 | 0 | 2 | - | 0 | 2 | 0 | 0 | 2 |  | 1 | 0 | 0 | 0 | 1 |  | 5 |
| \% Single-Unit Trucks | 0\% | 0\% | 0\% 0\% | \% | 0\% | - | 0\% | 1.9\% | 0\% 0\% | 0\% | 1.1\% | - | 0\% | 0.9\% | 0\% 0\% | \% | 0.8\% |  | 1.2\% | 0\% | 0\% 0\% | 0\% | 0.3\% |  | 0.5\% |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 |
| \% Articulated Trucks | 0\% | 0\% | 0\% 0\% | \% | 0\% | - | 0\% | 0\% | 0\% 0 |  | 0\% | - | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% | - | 0\% |
| Buses | 1 | 0 | 0 | 0 | 1 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 1 | 0 | 0 | 1 |  | 0 | 0 | 1 | 0 | 1 | - | 3 |
| \% Buses | 2.1\% | 0\% | 0\% 0\% | \% | 0.5\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% | - | 0\% | 0.5\% | 0\% 0\% | \% | 0.4\% |  | 0\% | 0\% | 2.0\% 0 |  | 0.3\% |  | 0.3\% |
| Bicycles on Road | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 |
| \% Bicycles on Road | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0\% | 0\% | 0\% 0\% |  | 0\% |  | 0\% | 0\% | 0\% 0 |  | 0\% |  | 0\% |
| Pedestrians | - | - | - | - | - | 0 | - | - | - - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Pedestrians | - | - | - | - | - | - | - | - | - - | - | - |  | - | - | - | - | - |  | - | - | - | - | - | - | - |
| Bicycles on Crosswalk | - | - | - | - | - | 0 | - | - | - - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 |  |
| \% Bicycles on Crosswalk | - | - | - | - | - |  | - | - | - - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |

[^1][N] S Genesee Rd
Total: 650
In: 339
Out: 311


Out: 258 In: 261
Total: 519
[S] S Genesee Rd

| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay，s／veh | 13.8 |
| Intersection LOS | B |


| Movement | EBU | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  |  | $\uparrow$ | 「 |  | $\uparrow$ | 「 |  | $\uparrow$ | 「 |  | $\uparrow$ |
| Traffic Vol，veh／h | 1 | 49 | 74 | 20 | 25 | 117 | 75 | 39 | 144 | 17 | 46 | 172 |
| Future Vol，veh／h | 1 | 49 | 74 | 20 | 25 | 117 | 75 | 39 | 144 | 17 | 46 | 172 |
| Peak Hour Factor | 0.69 | 0.69 | 0.69 | 0.69 | 0.89 | 0.89 | 0.89 | 0.82 | 0.82 | 0.82 | 0.74 | 0.74 |
| Heavy Vehicles，\％ | 3 | 3 | 3 | 3 | 2 | 2 | 2 | 1 | 1 | 1 | 3 | 3 |
| Mvmt Flow | 1 | 71 | 107 | 29 | 28 | 131 | 84 | 48 | 176 | 21 | 62 | 232 |
| Number of Lanes | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 0 | 1 | 1 | 0 | 1 |
| Approach | EB |  |  |  | WB |  |  | NB |  |  | SB |  |
| Opposing Approach | WB |  |  |  | EB |  |  | SB |  |  | NB |  |
| Opposing Lanes | 2 |  |  |  | 2 |  |  | 2 |  |  | 2 |  |
| Conflicting Approach Left | SB |  |  |  | NB |  |  | EB |  |  | WB |  |
| Conflicting Lanes Left | 2 |  |  |  | 2 |  |  | 2 |  |  | 2 |  |
| Conflicting Approach Right | NB |  |  |  | SB |  |  | WB |  |  | EB |  |
| Conflicting Lanes Right | 2 |  |  |  | 2 |  |  | 2 |  |  | 2 |  |
| HCM Control Delay | 13.4 |  |  |  | 12.1 |  |  | 14.1 |  |  | 14.9 |  |
| HCM LOS | B |  |  |  | B |  |  | B |  |  | B |  |


| Lane | NBLn1 | NBLn2 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left，\％ | $21 \%$ | $0 \%$ | $40 \%$ | $0 \%$ | $18 \%$ | $0 \%$ | $21 \%$ | $0 \%$ |
| Vol Thru，\％ | $79 \%$ | $0 \%$ | $60 \%$ | $0 \%$ | $82 \%$ | $0 \%$ | $79 \%$ | $0 \%$ |
| Vol Right，\％ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 183 | 17 | 124 | 20 | 142 | 75 | 218 | 92 |
| LT Vol | 39 | 0 | 49 | 0 | 25 | 0 | 46 | 0 |
| Through Vol | 144 | 0 | 75 | 0 | 117 | 0 | 172 | 0 |
| RT Vol | 0 | 17 | 0 | 20 | 0 | 75 | 0 | 92 |
| Lane Flow Rate | 223 | 21 | 180 | 29 | 160 | 84 | 295 | 124 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util（X） | 0.427 | 0.035 | 0.363 | 0.05 | 0.315 | 0.148 | 0.537 | 0.198 |
| Departure Headway（Hd） | 6.891 | 6.068 | 7.277 | 6.377 | 7.112 | 6.307 | 6.677 | 5.857 |
| Convergence，Y／N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 524 | 593 | 496 | 565 | 507 | 571 | 545 | 617 |
| Service Time | 4.603 | 3.78 | 4.997 | 4.077 | 4.824 | 4.019 | 4.377 | 3.557 |
| HCM Lane V／C Ratio | 0.426 | 0.035 | 0.363 | 0.051 | 0.316 | 0.147 | 0.541 | 0.201 |
| HCM Control Delay | 14.6 | 9 | 14.1 | 9.4 | 13.1 | 10.1 | 16.9 | 10 |
| HCM Lane LOS | B | A | B | A | B | B | C | A |
| HCM 95th－tile Q | 2.1 | 0.1 | 1.6 | 0.2 | 1.3 | 0.5 | 3.2 | 0.7 |

Intersection
Intersection Delay, s/veh
Intersection LOS

| Movement | SBR |
| :--- | ---: |
| Laneffonfigurations | 「 |
| Traffic Vol, veh/h | 92 |
| Future Vol, veh/h | 92 |
| Peak Hour Factor | 0.74 |
| Heavy Vehicles, \% | 3 |
| Mvmt Flow | 124 |
| Number of Lanes | 1 |
| Approach |  |
| Opposing Approach |  |
| Opposing Lanes |  |
| Conflicting Approach Left |  |
| Conflicting Lanes Left |  |
| Conflicting Approach Right |  |
| Conflicting Lanes Right |  |
| HCM Control Delay |  |
| HCM LOS |  |

Intersection: 1: Genesee Road \& Lapeer Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | ULT | R | LT | R | LT | R | LT | R |
| Maximum Queue (ft) | 71 | 49 | 85 | 48 | 77 | 25 | 105 | 62 |
| Average Queue (ft) | 32 | 10 | 34 | 20 | 38 | 8 | 47 | 24 |
| 95th Queue (ft) | 55 | 32 | 60 | 37 | 65 | 24 | 84 | 45 |
| Link Distance (ft) | 608 |  | 717 |  | 594 |  | 609 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 140 |  | 165 |  | 175 |
| Storage Bay Dist (ft) |  |  |  |  |  |  |  | 145 |

Network Summary
Network wide Queuing Penalty: 0

| Intersection |  |
| :--- | ---: |
| Intersection Delay，s／veh | 29.9 |
| Intersection LOS | D |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | $\uparrow$ | 「 |  | $\uparrow$ | 「 |  | 4 | 「＇ |  | $\uparrow$ | 「 |
| Traffic Vol，veh／h | 62 | 152 | 34 | 34 | 138 | 62 | 26 | 280 | 34 | 107 | 268 | 66 |
| Future Vol，veh／h | 62 | 152 | 34 | 34 | 138 | 62 | 26 | 280 | 34 | 107 | 268 | 66 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.83 | 0.83 | 0.83 | 0.87 | 0.87 | 0.87 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles，\％ | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Mvmt Flow | 67 | 165 | 37 | 41 | 166 | 75 | 30 | 322 | 39 | 116 | 291 | 72 |
| Number of Lanes | 0 | 1 | 1 | 0 | 1 | 1 | 0 | 1 | 1 | 0 | 1 | 1 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 2 |  |  | 2 |  |  | 2 |  |  | 2 |  |  |
| HCM Control Delay | 20.2 |  |  | 17.4 |  |  | 30.9 |  |  | 41.9 |  |  |
| HCM LOS | C |  |  | C |  |  | D |  |  | E |  |  |


| Lane | NBLn1 | NBLn2 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left，\％ | $8 \%$ | $0 \%$ | $29 \%$ | $0 \%$ | $20 \%$ | $0 \%$ | $29 \%$ | $0 \%$ |
| Vol Thru，\％ | $92 \%$ | $0 \%$ | $71 \%$ | $0 \%$ | $80 \%$ | $0 \%$ | $71 \%$ | $0 \%$ |
| Vol Right，\％ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 306 | 34 | 214 | 34 | 172 | 62 | 375 | 66 |
| LT Vol | 26 | 0 | 62 | 0 | 34 | 0 | 107 | 0 |
| Through Vol | 280 | 0 | 152 | 0 | 138 | 0 | 268 | 0 |
| RT Vol | 0 | 34 | 0 | 34 | 0 | 62 | 0 | 66 |
| Lane Flow Rate | 352 | 39 | 233 | 37 | 207 | 75 | 408 | 72 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util（X） | 0.775 | 0.078 | 0.555 | 0.079 | 0.493 | 0.161 | 0.89 | 0.139 |
| Departure Headway（Hd） | 7.937 | 7.171 | 8.593 | 7.716 | 8.568 | 7.738 | 7.859 | 6.99 |
| Convergence，Y／N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 457 | 499 | 419 | 464 | 421 | 463 | 461 | 512 |
| Service Time | 5.69 | 4.923 | 6.349 | 5.471 | 6.325 | 5.495 | 5.608 | 4.739 |
| HCM Lane V／C Ratio | 0.77 | 0.078 | 0.556 | 0.08 | 0.492 | 0.162 | 0.885 | 0.141 |
| HCM Control Delay | 33.2 | 10.5 | 21.6 | 11.1 | 19.4 | 12 | 47.3 | 10.9 |
| HCM Lane LOS | D | B | C | B | C | B | E | B |
| HCM 95th－tile Q | 6.7 | 0.3 | 3.3 | 0.3 | 2.7 | 0.6 | 9.6 | 0.5 |

Intersection: 1: Genesee Road \& Lapeer Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | LT | R | LT | R | LT | R | LT | R |
| Maximum Queue (ft) | 95 | 30 | 96 | 52 | 166 | 34 | 187 | 90 |
| Average Queue (ft) | 50 | 14 | 41 | 20 | 70 | 13 | 84 | 24 |
| 95th Queue (ft) | 83 | 31 | 71 | 41 | 127 | 31 | 152 | 59 |
| Link Distance (ft) | 608 |  | 717 |  | 59 |  | 609 |  |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  | 165 |  | 175 |  | 145 |
| Storage Bay Dist (ft) |  | 140 |  | 165 | 1 |  | 2 |  |

Network Summary
Network wide Queuing Penalty: 1

## Lapeer Road and S. Genesee Road - AM Peak

T. Rodel - C:\Users\902JAM

File View Help


## Lapeer Road and S. Genesee Road - PM Peak



| Intersection |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Lapeer Road and S. Genesse Road |  |  |  |  |
| Opinion of Probable Cost |  |  | By: Rowe PSC <br> Date: 5/24/2021 |  |
|  |  |  |  |  |
| PAY ITEM DESCRIPTION | ESTIMATED QUANTITY | UNIT | UNIT PRICE | AMOUNT |
| Mobilization (10\%) | 1 | LSUM | \$99,500.00 | \$99,500.00 |
| Sidewalk, Rem | 24 | Syd | \$10.00 | \$240.00 |
| Pavt, Rem | 6700 | Syd | \$10.00 | \$67,000.00 |
| Curb and Gutter, Rem | 1385 | Ft | \$10.00 | \$13,850.00 |
| Embankment, CIP | 3000 | Cyd | \$15.00 | \$45,000.00 |
| Excavation, Earth | 2000 | Cyd | \$10.00 | \$20,000.00 |
| Aggregate Base | 2500 | Ton | \$21.00 | \$52,500.00 |
| Shoulder, Cl II | 130 | Ton | \$25.00 | \$3,250.00 |
| HMA, 4E10 | 700 | Ton | \$85.00 | \$59,500.00 |
| HMA Approach | 175 | Ton | \$50.00 | \$8,750.00 |
| Conc Pavt, Nonreinf, 9 inch | 3000 | Syd | \$45.00 | \$135,000.00 |
| Joint, Contraction, Cp | 3000 | Ft | \$10.00 | \$30,000.00 |
| Joint, Expansion, E2 | 200 | Ft | \$25.00 | \$5,000.00 |
| Joint, Expansion, E3 | 1300 | Ft | \$15.00 | \$19,500.00 |
| Curb and Gutter, Conc, Det B1 | 2200 | Ft | \$25.00 | \$55,000.00 |
| Curb and Gutter, Conc, Det D1 | 220 | Ft | \$25.00 | \$5,500.00 |
| Curb, Conc. Det E1 | 100 | Ft | \$25.00 | \$2,500.00 |
| Sidewalk, Conc, 4 inch | 220 | Stt | \$5.00 | \$1,100.00 |
| Conc Pavt, Decorative Colored, 9 inch | 6000 | Sft | \$12.50 | \$75,000.00 |
| Turf Establishment, Performance | 5000 | Syd | \$5.00 | \$25,000.00 |
| MOT | 1 | LSUM | \$70,000.00 | \$70,000.00 |
| Pavement Markings | 1 | LSUM | \$15,000.00 | \$15,000.00 |
| Signing | 1 | LSUM | \$20,000.00 | \$20,000.00 |
| Drainage | 1 | LSUM | \$100,000 | \$100,000.00 |
| TOTAL \$928,190 |  |  |  |  |

## CONTINGENCY (20\%) <br> ESTIMATED TOTAL CONSTRUCTION COST (YEAR 2021) <br> 3\% ANNUAL INFLATION 2021 TO 2025 <br> ESTIMATED TOTAL CONSTRUCTION COST (YEAR 2025) <br> \$185,638.0 <br> \$1,113,828.0 <br> \$139,795.23 <br> \$1,253,623.23

* Notes

6" Agg base used for driveways and shoulders, 8 " used for roadway, 12 " used for islands ( $10 \%$ of total added) Asphalt shoulder gravel thickness was calculated using a 5" depth
Assumed 5" HMA thickness for approach and 8" thick for HMA roadway
B1 curb was calculated by adding the splitter islands along with the roadways, minus the radius for the roundabout E1 curb was calculated for the inner roundabout curb
E2 joint expansion was caclulated for the outer radius of the roundabout

E3 joint expansion was caclulated using the radius of curves from the $B / C$ of the roundabout and splitter islands D1 curb was calculated for the truck apron
Drainage includes enclosed storm for all curbed areas, spillways, and underdrain

MOT taken as approximately ~8\% of total before mobilization; based on previous roundabout projects
ROW acqusition costs not inlcuded
Joints assumed for central island/splitter islands

Pavement and curb removals include the assumption the south east island at the intersection will be removed.

NUMBER OF CRASHES OR INJURED PERSONS.


REMARKS:

```
Lapeer Road and Genesee Road Genesee County Roundabout Study 1495003, 1520305 3.5075757, 4.5148824
Roundabout
```


# COMPUTED BENEFITS DERIVED THROUGH CRASH REDUCTION 

TOR 2021
Project: Lapeer Road and Genesee Road
Prepared By: ROWE Professional Services Company

City/Twp. City of Burton
County Genesee County

PR: 1495003, 1520305 PR MP Range: 3.5075757, 4.5148824

The method of evaluating crash costs, used below, is given on page 67 of Roy Jorgensen's report of Highway Safety Improvement Criteria, 1966 edition. This same method is given in the Bureau of Public Roads IM21-3-67. In 1994 we have adapted the Q formula to blend Fatalities and A -injuries only. In the following analysis the costs provided by the National Safety Council are :

2019 NSC VALUES:

| Death | $\$ 1,659,000$ | $=$ FATCOST |
| :--- | ---: | :--- |
| Disabling (A) injury: | $\$ 96,200$ | $=$ ACOST |
| B-injury: | $\$ 27,800=$ BCOST |  |
| PDO and/or Minor Injury Crash: | $\$ 12,200$ | $=$ PDOCOST |
|  |  |  |
| BTOTAL $=$ ADTa $/$ ADTb $\times[(Q \times R 1)+(B C O S T \times R 2)+(P D O C O S T \times R 3)]$ |  |  |

WHERE:

BTOTAL $=$ Total Benefit in Dollars Over Years Used \$459,320
ADTa $=\quad$ Average traffic volume after the improvement 1.1
ADTb $=\quad$ Average traffic volume before the improvement 1.0
R1 $=$ Reduction in fatalities and A-Injuries Combined. 0.8
R2 $=$ Reduction in B-Injury crashes: 1.1
R3 $=$ Reduction in PDO and C-injury crashes: 11.4
$\mathrm{Q}=[$ FATCOST $+((\mathrm{I} / \mathrm{F}) \times$ INJCOST $)] /[1+(\mathrm{I} / \mathrm{F})]$
$=\quad[1,659,000+(6.10 \times 96,200)] /[1+6.10]$
\$316,400
for AREA TYPE "Urban"
I/F =
6.10

| Q-Reference | Q | A-Inuries | Fatalities | $\mathrm{I} / \mathrm{F}$ |
| :--- | ---: | ---: | ---: | ---: |
| RURAL | $\$ 363,900$ | 6,072 | 1,255 | 4.84 |
| URBAN | $\$ 316,400$ | 9,902 | 1,624 | 6.10 |
| BETWEEN | $\$ 334,900$ | 15,974 | 2,879 | 5.55 |

Data from Safety Programs Unit
5-Year Statewide Non-Trunkline Crash Figures Used.
(*From 1-1-2015 Through 12-31-2019).

Time of Return (T.O.R.) is based on ....
NOINFB $=$ No-Inflation Annual Benefit=BTOTAL/years
With an inflation rate of $\qquad$
$B=A n n u a l$ Benefit=Present Value (with Inflation)

C $=$ Project Cost

TOR=C/B=COST/ANNUAL BENEFIT=
2.50\%
\$117,594
\$1,253,623
10.66
H. Lapeer and S. Genesee

1. 2045 AM Peak Hour No Build
2. 2045 PM Peak Hour No Build
3. 2045 AM Peak Hour Roundabout
4. 2045 PM Peak Hour Roundabout

[^0]:    * $95^{\text {th }}$ percentile queue length.

[^1]:    *Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

