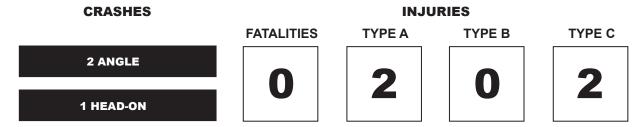
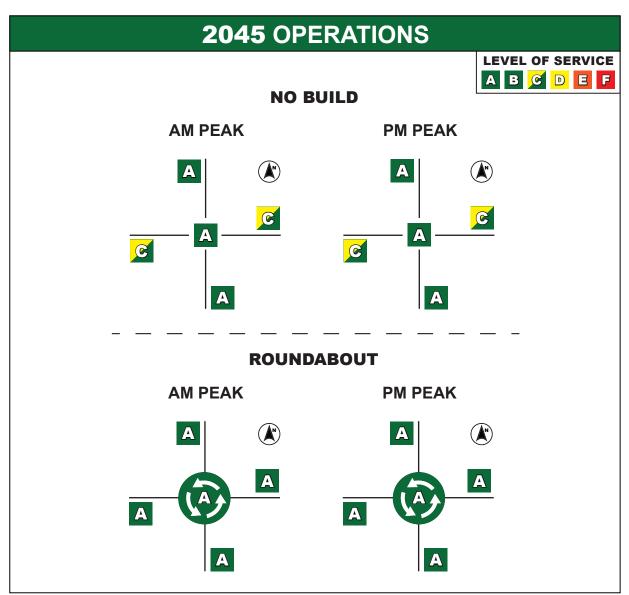
CLIO ROAD/WELCH BLVD. & DAYTON STREET

2015-2019 CRASH DATA





Opinion of probable cost for single-lane roundabout

\$1.33 MILLION



D. Clio Road/Welch Boulevard and Dayton Street

The intersection of Clio Road/Welch Boulevard and Dayton Street was included in the early preliminary engineering phase with support from the City of Flint due to intersection operations. This intersection is a secondary analysis Tier Three intersection.

During the skim analysis, it was observed that there were two angle crashes and one head on left turn crash over the 5-year period. These crashes resulted in zero fatalities, two type A injuries, zero type B injuries, and two type C injuries at the intersection.

No-Build Conditions

The intersection of Clio Road/Welch Boulevard and Dayton Street is a signalized intersection. Clio Road is a north/south road with five lanes, two lanes in each direction with a center left-turn lane. Welch Boulevard is a northwest/southeast road with four lanes, two lanes in each direction. At the intersection with Dayton Street, the approach has an exclusive left-turn lane, two through lanes, and a right-turn slip lane. Dayton Street is an east/west road, with two lanes in each direction. At the intersection with Clio Road/Welch Boulevard, the approaches present an exclusive left-turn lane and a shared through/right-turn lane. Traffic signals are supported by mast arms. There are pedestrian crossings at all legs of the intersection.

A commercial building is present in the northwest quadrant. In the northeast quadrant is the Clio Road Market. The southeast quadrant has a residential vacant field. The Genesee County Free Medical Clinic is located in the southwest quadrant. There are overhead electrical cables crossing through the intersection on the west and north sides of the intersection.

An aerial of the existing intersection can be seen in Figure 5.

Page 22



Figure 5: Aerial view of Clio Road/Welch Boulevard and Dayton Street

An operational analysis of the no-build condition was completed for the intersection using the 2045 forecast traffic volumes. The results of the analysis for the future no-build condition reveals all approaches and movements of the intersection operate at LOS C during the AM and PM peak hours.

The 95th percentile queue lengths were reviewed at the intersection and the results showed all approaches experienced a maximum queue length of 95 feet (6 vehicles) during the AM peak hour and 136 feet (9 vehicles) during the PM peak hour.

The operational analysis results for the future no-build conditions are presented in Table 12.

Table 12: Operational Analysis for 2045 No-Build Condition

		- 1			
Intersection	Annyoosh	AIV	l Peak	PM	l Peak
intersection	Approach	Delay/LOS	Queue (veh)*	Delay/LOS	Queue (veh)*
	Eastbound	24.6/C	2 (30 ft)	25.4/C	2 (34 ft)
Clic Bood (Moleh Bouleyard and	Westbound	24.9/C	1 (22 ft)	25.8/C	2 (24 ft)
Clio Road/Welch Boulevard and	Northbound	6.2/A	4 (67 ft)	7.1/A	7 (116 ft)
Dayton Street	Southbound	6.5/A	6 (95 ft)	7.3/A	9 (136 ft)
	Overall	7.8/A		8.8/A	

^{* 95&}lt;sup>th</sup> percentile queue length.

Roundabout Conditions

The proposed roundabout configuration for the intersection of Clio Road/Welch Boulevard and Dayton Street is a single-lane circulating. The proposed single-lane roundabout configuration will likely fit inside the existing right-of-way. The proposed inscribed diameter for the concept roundabout

is 100 feet. The driveway configuration for the Clio Road Market should be investigated due to the existing driveways closeness to the proposed roundabout entry/exit radius.

An operational analysis for the single-lane roundabout (build) condition was completed for the intersection using 2045 forecast traffic volumes. The results of the analysis for the roundabout (build) condition reveals all approaches and movements of the intersection operate at LOS A during the AM and PM peak hours.

The 95th percentile queue lengths were reviewed at the intersection and the results showed all approaches experienced a maximum queue length of one (1) vehicle during the AM peak hour and three (3) vehicles during the PM peak hour.

The operation analysis for the future roundabout (build) conditions are presented in Table 13.

Table 13: Operational Analysis for 2045 Roundabout (Build) Condition

Intersection	Ammuooch	AN	l Peak	PM	l Peak
intersection	Approach	Delay/LOS	Queue (veh)*	Delay/LOS	Queue (veh)*
	Eastbound	2.2/A	0.0	4.2/A	0.1
Clip Boad (Wolch Blud and Wost	Westbound	3.8/A	0.1	4.7/A	0.2
Clio Road/Welch Blvd and West	Northbound	5.8/A	0.7	9.3/A	2.9
Dayton Street (Single-lane RAB)	Southbound	6.7/A	1.2	9.8/A	3.3
	Overall	6.1/A		9.2/A	

^{* 95&}lt;sup>th</sup> percentile queue length.

Opinion of probable cost were developed for a single-lane roundabout. The total probable cost is \$1.33 million in year 2025 dollars. The probable cost includes a 20 percent contingency and 3 percent inflation. Not included in this fee are the potential costs to relocate any utilities to accommodate the proposed layout. A full breakdown along with all the assumptions can be found in Appendix 3.

Potential funding sources for this improvement could include regular road improvement funding or an earmark.

Recommendation

A roundabout appears feasible at this location but will require coordination with the Clio Road Market for driveway reconfiguration.



PLAN DATE:

ROWE PROFESSIONAL

SERVICES COMPANY



CLIO ROAD/WELCH BLVD AND DAYTON STREET ROUNDABOUT CONCEPT



GENESEE COUNTY RO ROUNDABOUT

FIGURE NO. 25

OAD COMMISSION CONCEPTS

AND WEST DAYTON STREET OUT CONCEPT

CLIO ROAD/WELCH BLVD.

JOB No: 19C0262

Tue Mar 9, 2021

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	W Dayt Eastbou						W Dayt Westbo					Clio R						Clio Ro Southb						
Time	L	T	R	U	App	Ped*	L	T	R	U	App Ped*	L	T	R	J	Арр	Ped*	L	T	R	U	Арр	Ped*	Int
2021-03-09 7:00AM	0	0	0	0	0	0	0	0	1	0	1 0	0	19	0	0	19	0	1	24	0	0	25	0	45
7:15AM	0	0	1	0	1	0	0	0	2	0	2 0	1	28	0	0	29	0	3	39	0	0	42	0	74
7:30AM	0	2	2	0	4	0	1	1	4	0	6 0	1	33	1	0	35	0	2	47	1	0	50	0	95
7:45AM	0	0	0	0	0	0	0	0	2	0	2 0	0	27	0	0	27	0	5	55	0	0	60	0	89
Hourly Total	0	2	3	0	5	0	1	1	9	0	11 0	2	107	1	0	110	0	11	165	1	0	177	0	303
8:00AM	0	0	1	0	1	0	0	1	6	0	7 0	0	31	1	0	32	0	4	57	1	0	62	0	102
8:15AM	0	0	3	0	3	0	2	0	3	0	5 0	4	44	0	0	48	1	7	54	0	0	61	0	117
8:30AM	1	0	0	0	1	0	1	0	0	0	1 0	2	50	1	0	53	0	2	60	1	0	63	0	118
8:45AM	0	0	4	0	4	0	1	0	2	0	3 1	. 1	46	0	0	47	0	0	62	1	0	63	1	117
Hourly Total	1	0	8	0	9	0	4	1	11	0	16 1	. 7	171	2	0	180	1	13	233	3	0	249	1	454
4:00PM	1	0	4	0	5	0	1	0	5	0	6 0	5	105	3	0	113	0	5	114	1	0	120	1	244
4:15PM	1	0	2	0	3	1	2	0	11	0	13 0	3	115	2	0	120	0	4	113	0	0	117	2	253
4:30PM	1	0	9	0	10	0	4	1	1	0	6 1	. 1	97	1	0	99	0	6	103	0	0	109	0	224
4:45PM	2	0	3	0	5	0	1	2	11	0	14 0	1	102	0	0	103	0	6	110	0	0	116	1	238
Hourly Total	5	0	18	0	23	1	8	3	28	0	39 1	. 10	419	6	0	435	0	21	440	1	0	462	4	959
5:00PM	1	0	3	0	4	0	4	0	13	0	17 0	3	104	2	0	109	0	8	97	1	0	106	0	236
5:15PM	1	3	3	0	7	0	2	2	9	0	13 0	1	80	7	0	88	0	5	111	0	0	116	1	224
5:30PM	0	0	3	0	3	0	5	1	6	0	12 0	3	101	7	0	111	0	6	103	2	0	111	1	237
5:45PM	1	1	1	0	3	0	2	1	10	0	13 0	1	82	4	0	87	0	5	93	1	0	99	3	202
Hourly Total	3	4	10	0	17	0	13	4	38	0	55 0	8	367	20	0	395	0	24	404	4	0	432	5	899
Total	9	6	39	0	54	1	26	9	86	0	121 2	27	1064	29	0 1	1120	1	69	1242	9	0	1320	10	2615
% Approach	16.7%	11.1%	72.2% (0%	-	-	21.5%	7.4%	71.1% (1%		2.4%	95.0%	2.6% 09	6	-	-	5.2%	94.1%).7% ()%	-	-	-
% Total	0.3%	0.2%	1.5% (0%	2.1%	-	1.0%	0.3%	3.3% (1%	4.6% -	1.0%	40.7%	1.1% 09	6 4 2	2.8%	-	2.6%	47.5%	0.3% ()% 5	60.5%	-	-
Lights	8	5	39	0	52	-	26	8	79	0	113 -	25	1051	27	0 1	1103	-	59	1226	9	0	1294	-	2562
% Lights	88.9%	83.3%	100% (0% 9	6.3%	-	100% 8	38.9%	91.9% (% 9	93.4% -	92.6%	98.8%	93.1% 09	6 98	3.5%	-	85.5%	98.7% 1	.00% ()% 9	8.0%	-	98.0%
Single-Unit Trucks	0	0	0	0	0	-	0	0	1	0	1 -	. 0	6	2	0	8	-	0	11	0	0	11	-	20
% Single-Unit Trucks	0%	0%	0% (0%	0%	-	0%	0%	1.2% (1%	0.8% -	0%	0.6%	6.9% 09	6 O).7%	-	0%	0.9%	0% ()%	0.8%	-	0.8%
Articulated Trucks	0	0	0	0	0	-	0	0	0	0	0 -	. 0	0	0	0	0	-	0	2	0	0	2	-	2
% Articulated Trucks	0%	0%	0% (0%	0%	-	0%	0%	0% 0	1%	0% -	0%	0%	0% 09	6	0%	-	0%	0.2%	0% ()%	0.2%	-	0.1%
Buses	1	1	0	0	2	-	0	1	6	0	7 -	. 2	7	0	0	9	-	10	3	0	0	13	-	31
% Buses	11.1%	16.7%	0% (0%	3.7%	-	0%	11.1%	7.0% (1%	5.8% -	7.4%	0.7%	0% 09	6 C).8%	-	14.5%	0.2%	0% (0%	1.0%	-	1.2%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0 -	. 0	0	0	0	0	-	0	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0% (0%	0%	-	0%	0%	0% 0	%	0% -	- 0%	0%	0% 09	6	0%	-	0%	0%	0% ()%	0%	-	0%
Pedestrians	-	-	-	-	-	1	-	-	-	-	- 2	! -	-	-	-	-	1	-	-	-	-	-	10	
% Pedestrians	-	-	-	-	- 1	100%	-	-	-	-	- 100%	-	-	-	-	- :	100%	-	-	-	-	- 1	.00%	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	- 0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	0%	-	-	-	-	- 0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-

 $^{^*}$ Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Tue Mar 9, 2021

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses,

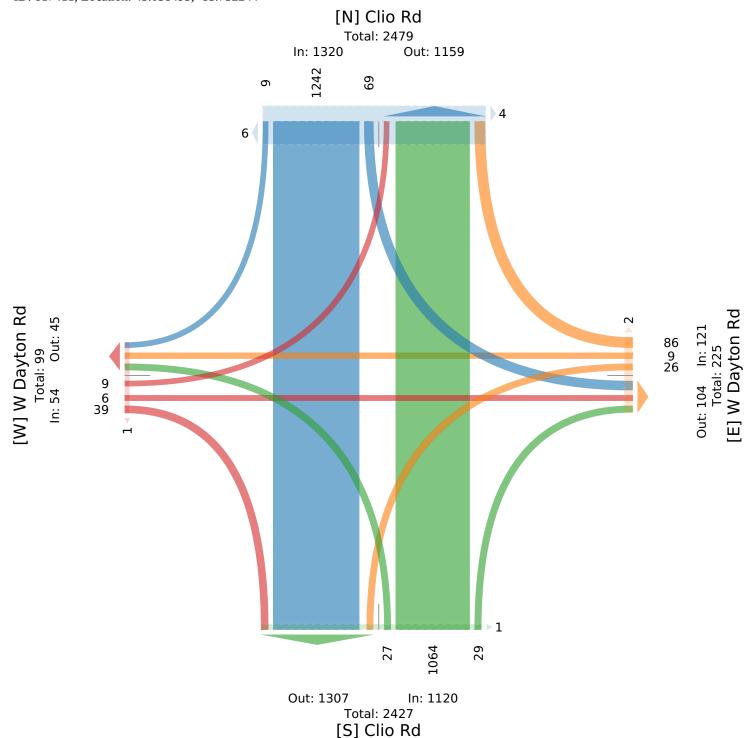
Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



625 Forest Edge Drive, Vernon Hills, IL, 60061, US



Tue Mar 9, 2021

AM Peak (8 AM - 9 AM)

All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg	W Day		Rd				W Day						Clio R						Clio Rd						
Direction	Eastbou	und					Westbo	und					Northb	ound					Southbo	ound					
Time	L	T	R	U	App I	ed*	L	T	R		App	Ped*	L	T	R	U	App	Ped*	L	T	R	U	App	Ped*	Int
2021-03-09 8:00AM	0	0	1	0	1	0	0	1	6	0	7	0	0	31	1	0	32	0	4	57	1	0	62	0	102
8:15AM	0	0	3	0	3	0	2	0	3	0	5	0	4	44	0	0	48	1	7	54	0	0	61	0	117
8:30AM	1	0	0	0	1	0	1	0	0	0	1	0	2	50	1	0	53	0	2	60	1	0	63	0	118
8:45AM	0	0	4	0	4	0	1	0	2	0	3	1	1	46	0	0	47	0	0	62	1	0	63	1	117
Total	1	0	8	0	9	0	4	1	11	0	16	1	7	171	2	0	180	1	13	233	3	0	249	1	454
% Approach	11.1%	0%	88.9% (0%	-	-	25.0%	6.3%	68.8% ()%	-	-	3.9%	95.0%	1.1%	0%	-	-	5.2% 9	93.6%	1.2%	0%	-	-	-
% Total	0.2%	0%	1.8%	0%	2.0%	-	0.9%	0.2%	2.4% ()%	3.5%	-	1.5%	37.7%	0.4%	0% 3	39.6%	-	2.9% !	51.3%	0.7%	0% 5	54.8%	-	-
PHF	0.250	-	0.500	-	0.563	-	0.500	0.250	0.458	-	0.571	-	0.438	0.855	0.500	-	0.849	-	0.464	0.940	0.750	-	0.988	-	0.962
Lights	0	0	8	0	8	-	4	1	9	0	14	-	7	167	2	0	176	-	10	229	3	0	242	-	440
% Lights	0%	0%	100% (0% 8	88.9%	-	100%	100%	81.8% ()% {	37.5%	-	100%	97.7%	100%	0% 9	97.8%	-	76.9% 9	98.3%	100%	0% 9	97.2%	-	96.9%
Single-Unit Trucks	0	0	0	0	0	-	0	0	0	0	0	-	0	3	0	0	3	-	0	2	0	0	2	-	5
% Single-Unit Trucks	0%	0%	0% (0%	0%	-	0%	0%	0% ()%	0%	-	0%	1.8%	0%	0%	1.7%	-	0%	0.9%	0% (0%	0.8%	-	1.1%
Articulated Trucks	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1	0	0	1	-	1
% Articulated Trucks	0%	0%	0% (0%	0%	-	0%	0%	0% ()%	0%	-	0%	0%	0%	0%	0%	-	0%	0.4%	0% (0%	0.4%	-	0.2%
Buses	1	0	0	0	1	-	0	0	2	0	2	-	0	1	0	0	1	-	3	1	0	0	4	-	8
% Buses	100%	0%	0% (0% 1	11.1%	-	0%	0%	18.2% ()% :	12.5%	-	0%	0.6%	0%	0%	0.6%	-	23.1%	0.4%	0% (0%	1.6%	-	1.8%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0% (0%	0%	-	0%	0%	0% ()%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0% (0%	0%	-	0%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	1	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	100%	-	-	-	-	-	100%	-	-	-	-	- 1	.00%	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	0%	-	-	-	-	-	0%	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Tue Mar 9, 2021

AM Peak (8 AM - 9 AM)

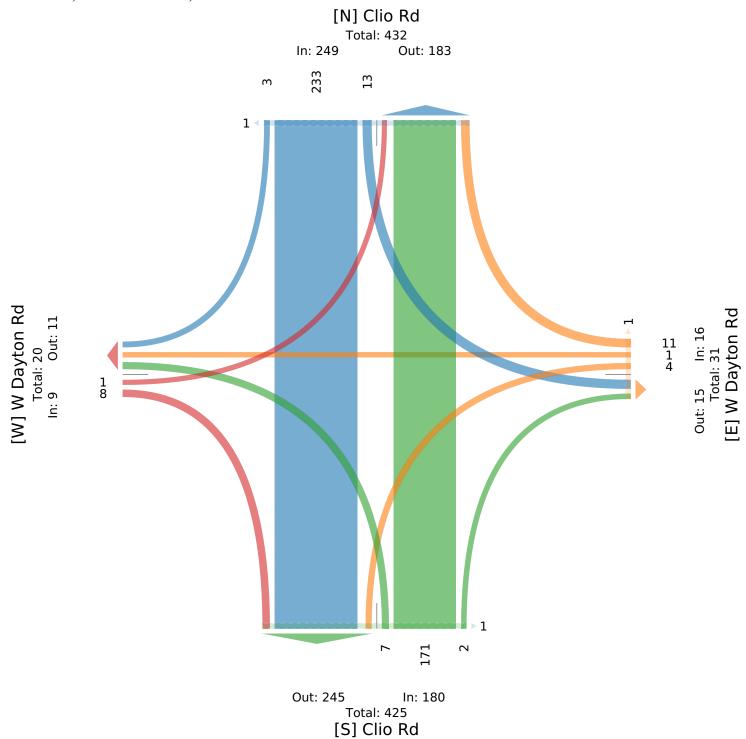
All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US



Tue Mar 9, 2021

PM Peak (4 PM - 5 PM) - Overall Peak Hour

All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg	W Dayt	on F	Rd				W Day	ton Rd					Clio Rd	l					Clio Rd	l					
Direction	Eastbou	ınd					Westbo	und					Northbo	ound					Southbo	ound					
Time	L	T	R	U	App	Ped*	L	T	R	U	App	Ped*	L	T	R	U	App P	ed*	L	T	R	U	App	Ped*	Int
2021-03-09 4:00PM	1	0	4	0	5	0	1	0	5	0	6	0	5	105	3	0	113	0	5	114	1	0	120	1	244
4:15PM	1	0	2	0	3	1	2	0	11	0	13	0	3	115	2	0	120	0	4	113	0	0	117	2	253
4:30PM	1	0	9	0	10	0	4	1	1	0	6	1	1	97	1	0	99	0	6	103	0	0	109	0	224
4:45PM	2	0	3	0	5	0	1	2	11	0	14	0	1	102	0	0	103	0	6	110	0	0	116	1	238
Total	5	0	18	0	23	1	8	3	28	0	39	1	10	419	6	0	435	0	21	440	1	0	462	4	959
% Approach	21.7% (0% 7	78.3% ()%	-	-	20.5%	7.7%	71.8%	0%	-	-	2.3%	96.3%	1.4% ()%	-	-	4.5% 9	95.2%	0.2% ()%	-	-	-
% Total	0.5% (0%	1.9% ()%	2.4%	-	0.8%	0.3%	2.9%	0%	4.1%	-	1.0%	43.7%	0.6% ()% 4	5.4%	-	2.2%	45.9%	0.1% ()% 4	18.2%	-	-
PHF	0.625	-	0.500	- ().575	-	0.500	0.375	0.636	-	0.696	-	0.500	0.911	0.500	- (0.906	-	0.875	0.965	0.250	- (0.963	-	0.948
Lights	5	0	18	0	23	-	8	3	27	0	38	-	9	415	5	0	429	-	19	437	1	0	457	-	947
% Lights	100% (0%	100% ()% 1	100%	-	100%	100%	96.4%	0% 9	97.4%	-	90.0%	99.0%	83.3% ()% 9	8.6%	-	90.5%	99.3%	100% ()% 9	98.9%	-	98.7%
Single-Unit Trucks	0	0	0	0	0	-	0	0	0	0	0	-	0	1	1	0	2	-	0	2	0	0	2	-	4
% Single-Unit Trucks	0% (0%	0% ()%	0%	-	0%	0%	0% (0%	0%	-	0%	0.2%	16.7% ()%	0.5%	-	0%	0.5%	0% ()%	0.4%	-	0.4%
Articulated Trucks	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Articulated Trucks	0% (0%	0% ()%	0%	-	0%	0%	0% (0%	0%	-	0%	0%	0% 0)%	0%	-	0%	0%	0% ()%	0%	-	0%
Buses	0	0	0	0	0	-	0	0	1	0	1	-	1	3	0	0	4	-	2	1	0	0	3	-	8
% Buses	0% (0%	0% ()%	0%	-	0%	0%	3.6%	0%	2.6%	-	10.0%	0.7%	0% 0)%	0.9%	-	9.5%	0.2%	0% ()%	0.6%	-	0.8%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Bicycles on Road	0% (0%	0% ()%	0%	-	0%	0%	0% (0%	0%	-	0%	0%	0% 0)%	0%	-	0%	0%	0% ()%	0%	-	0%
Pedestrians	-	-	-	-	-	1	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	4	
% Pedestrians	-	-	-	-	-	100%	-	-	-	-	-	100%	-	-	-	-	-	-	-	-	-	-	-	100%	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	_	0%	-	-	-	-	-	0%	-	-	-	-	-	-	-	-	-	-	-	0%	-

^{*}Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

Tue Mar 9, 2021

PM Peak (4 PM - 5 PM) - Overall Peak Hour

All Classes (Lights, Single-Unit Trucks, Articulated Trucks, Buses,

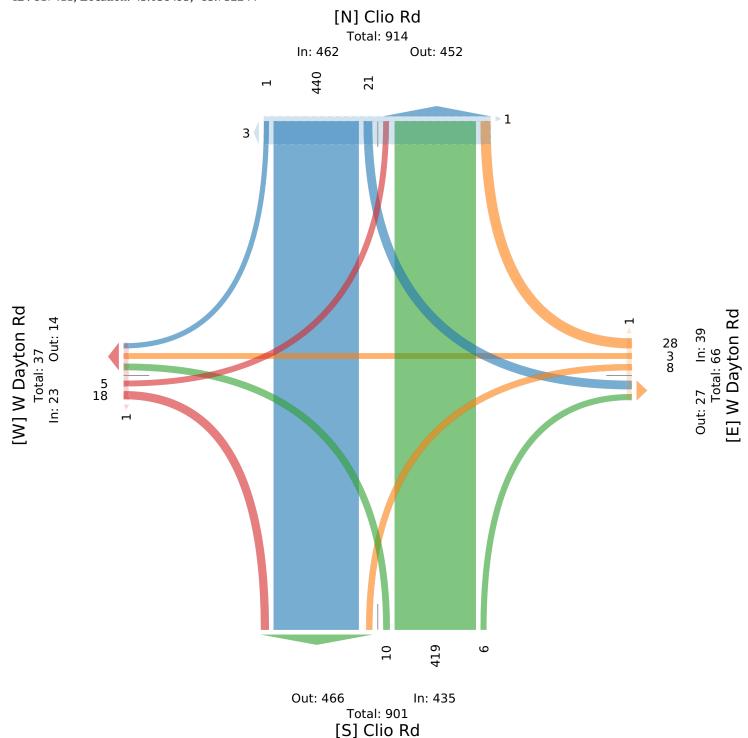
Pedestrians, Bicycles on Road, Bicycles on Crosswalk)

All Movements

ID: 817411, Location: 43.038493, -83.732244



Provided by: Gewalt Hamilton Associates Inc. 625 Forest Edge Drive, Vernon Hills, IL, 60061, US



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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	₽		7	₽		7	ተኈ		7	ħβ	
Traffic Volume (veh/h)	1	0	10	5	1	14	9	222	3	17	303	4
Future Volume (veh/h)	1	0	10	5	1	14	9	222	3	17	303	4
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	4707	No	4707	4707	No	4707	4070	No	4070	1050	No	1050
Adj Sat Flow, veh/h/ln	1737	1737	1737	1707	1707	1707	1870	1870	1870	1856	1856	1856
Adj Flow Rate, veh/h	2	0	17	8	2	23	11	261	0	18	319	4
Peak Hour Factor	0.60	0.60	0.60	0.60	0.60	0.60	0.85	0.85	0.85	0.95	0.95	0.95
Percent Heavy Veh, %	11	11	11	13	13	13	2	2	2	3	3	3
Cap, veh/h	362	0	331	365	26	303	712	2221	0.00	750	2228	28
Arrive On Green	0.22	0.00	0.22	0.22	0.22	0.22	0.63	0.63	0.00	0.63	0.63	0.63
Sat Flow, veh/h	1287	0	1472	1274	117	1348	1057	3647	0	1110	3566	45
Grp Volume(v), veh/h	2	0	17	8	0	25	11	261	0	18	158	165
Grp Sat Flow(s),veh/h/ln	1287	0	1472	1274	0	1465	1057	1777	0	1110	1763	1848
Q Serve(g_s), s	0.1	0.0	0.7	0.4	0.0	1.1	0.3	2.4	0.0	0.5	2.9	3.0
Cycle Q Clear(g_c), s	1.2	0.0	0.7	1.1	0.0	1.1	3.3	2.4	0.0	2.9	2.9	3.0
Prop In Lane	1.00	0	1.00	1.00	0	0.92	1.00	0004	0.00	1.00	4400	0.02
Lane Grp Cap(c), veh/h	362	0	331	365	0	330	712	2221		750	1102	1155
V/C Ratio(X)	0.01	0.00	0.05	0.02	0.00	0.08	0.02	0.12		0.02	0.14	0.14
Avail Cap(c_a), veh/h	362	1.00	331 1.00	365	0 1.00	330	712	2221	1.00	750 1.00	1102 1.00	1155
HCM Platoon Ratio	1.00	1.00	1.00	1.00	0.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00	1.00	1.00	1.00
Upstream Filter(I) Uniform Delay (d), s/veh	24.9	0.00	24.3	24.7	0.00	24.4	6.9	6.1	0.00	6.7	6.2	6.2
Incr Delay (d2), s/veh	0.0	0.0	0.3	0.1	0.0	0.4	0.0	0.1	0.0	0.7	0.2	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.0	0.0	0.5	0.0	0.0	0.7	0.0	1.4	0.0	0.0	1.8	1.9
Unsig. Movement Delay, s/veh		0.0	0.0	0.2	0.0	0.1	0.1	1.7	0.0	0.2	1.0	1.5
LnGrp Delay(d),s/veh	24.9	0.0	24.6	24.9	0.0	24.9	6.9	6.2	0.0	6.7	6.4	6.4
LnGrp LOS	Z4.5	Α	Z-4.0	24.5 C	Α	Z-4.5	Α	Α	0.0	Α	Α	Α
Approach Vol, veh/h		19			33			272	А		341	
Approach Delay, s/veh		24.6			24.9			6.2	А		6.5	
Approach LOS		C C			C C			A			A	
1.1											,,	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		56.0		24.0		56.0		24.0				
Change Period (Y+Rc), s		* 6		* 6		* 6		* 6				
Max Green Setting (Gmax), s		* 50		* 18		* 50		* 18				
Max Q Clear Time (g_c+l1), s		5.3		3.2		5.0		3.1				
Green Ext Time (p_c), s		1.8		0.0		2.0		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			7.8									
HCM 6th LOS			Α									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

Intersection: 1: Welch Blvd/Clio Road & Dayton Street

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR
Maximum Queue (ft)	6	50	15	38	14	86	47	40	121	65
Average Queue (ft)	0	7	2	6	1	30	7	6	50	17
95th Queue (ft)	4	30	10	22	7	67	28	26	95	48
Link Distance (ft)		421		475		520	520		578	578
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)	50		60		110			80		
Storage Blk Time (%)		0		0		0			1	
Queuing Penalty (veh)		0		0		0			0	

Network Summary

Network wide Queuing Penalty: 0

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1•		ሻ	₽		ሻ	∱ ∱		ሻ	∱ ∱	
Traffic Volume (veh/h)	7	0	23	10	4	36	13	545	8	27	572	1
Future Volume (veh/h)	7	0	23	10	4	36	13	545	8	27	572	1
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1856	1856	1856	1885	1885	1885	1885	1885	1885
Adj Flow Rate, veh/h	12	0	38	14	6	51	14	599	0	28	602	1
Peak Hour Factor	0.60	0.60	0.60	0.70	0.70	0.70	0.91	0.91	0.91	0.95	0.95	0.95
Percent Heavy Veh, %	0	0	0	3	3	3	1	1	1	1	1	1
Cap, veh/h	359	0	362	370	38	322	544	2239		544	2293	4
Arrive On Green	0.22	0.00	0.22	0.22	0.22	0.22	0.63	0.63	0.00	0.63	0.63	0.63
Sat Flow, veh/h	1368	0	1610	1359	168	1430	823	3676	0	826	3669	6
Grp Volume(v), veh/h	12	0	38	14	0	57	14	599	0	28	294	309
Grp Sat Flow(s),veh/h/ln	1368	0	1610	1359	0	1598	823	1791	0	826	1791	1884
Q Serve(g_s), s	0.6	0.0	1.5	0.7	0.0	2.3	0.6	6.0	0.0	1.3	5.9	5.9
Cycle Q Clear(g_c), s	2.9	0.0	1.5	2.2	0.0	2.3	6.5	6.0	0.0	7.3	5.9	5.9
Prop In Lane	1.00		1.00	1.00		0.89	1.00		0.00	1.00		0.00
Lane Grp Cap(c), veh/h	359	0	362	370	0	360	544	2239		544	1119	1178
V/C Ratio(X)	0.03	0.00	0.10	0.04	0.00	0.16	0.03	0.27		0.05	0.26	0.26
Avail Cap(c_a), veh/h	359	0	362	370	0	360	544	2239		544	1119	1178
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	26.1	0.0	24.6	25.5	0.0	24.9	8.2	6.8	0.0	8.4	6.7	6.7
Incr Delay (d2), s/veh	0.2	0.0	0.6	0.2	0.0	0.9	0.1	0.3	0.0	0.2	0.6	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.4	0.0	1.1	0.4	0.0	1.7	0.2	3.5	0.0	0.4	3.6	3.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	26.2	0.0	25.2	25.7	0.0	25.9	8.3	7.0	0.0	8.6	7.3	7.3
LnGrp LOS	С	Α	С	С	Α	С	Α	Α		Α	Α	Α
Approach Vol, veh/h		50			71			613	Α		631	
Approach Delay, s/veh		25.4			25.8			7.1			7.3	
Approach LOS		С			С			Α			Α	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		56.0		24.0		56.0		24.0				
Change Period (Y+Rc), s		* 6		* 6		* 6		* 6				
Max Green Setting (Gmax), s		* 50		* 18		* 50		* 18				
Max Q Clear Time (g_c+l1), s		8.5		4.9		9.3		4.3				
Green Ext Time (p_c), s		4.6		0.1		4.1		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			8.8									
HCM 6th LOS			Α									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR] is excluded from calculations of the approach delay and intersection delay.

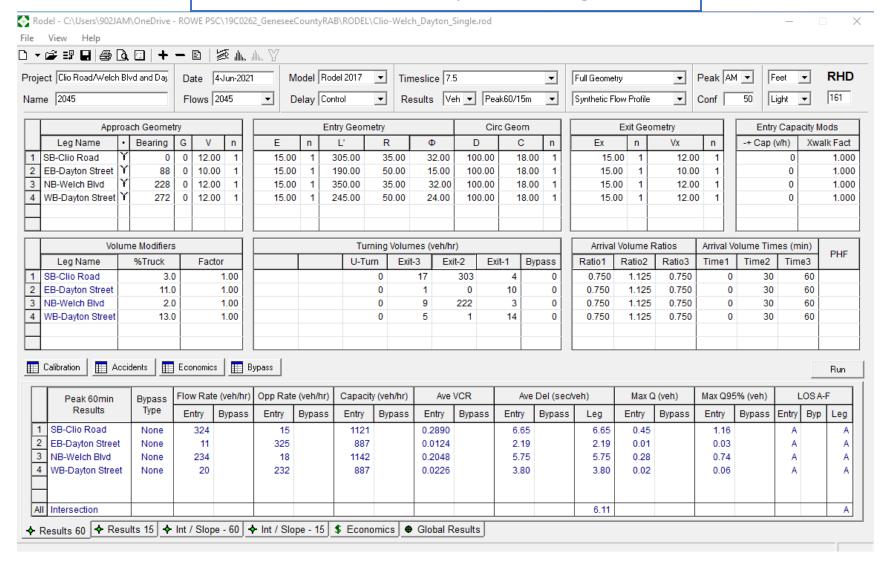
Intersection: 1: Welch Blvd/Clio Road & Dayton Street

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	43	31	18	37	20	132	97	96	155	92	
Average Queue (ft)	6	12	3	8	2	68	30	17	85	37	
95th Queue (ft)	27	34	15	24	10	116	69	53	136	76	
Link Distance (ft)		421		475		520	520		576	576	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	50		60		110			80			
Storage Blk Time (%)	1	0		0		1			5		
Queuing Penalty (veh)	0	0		0		0			1		

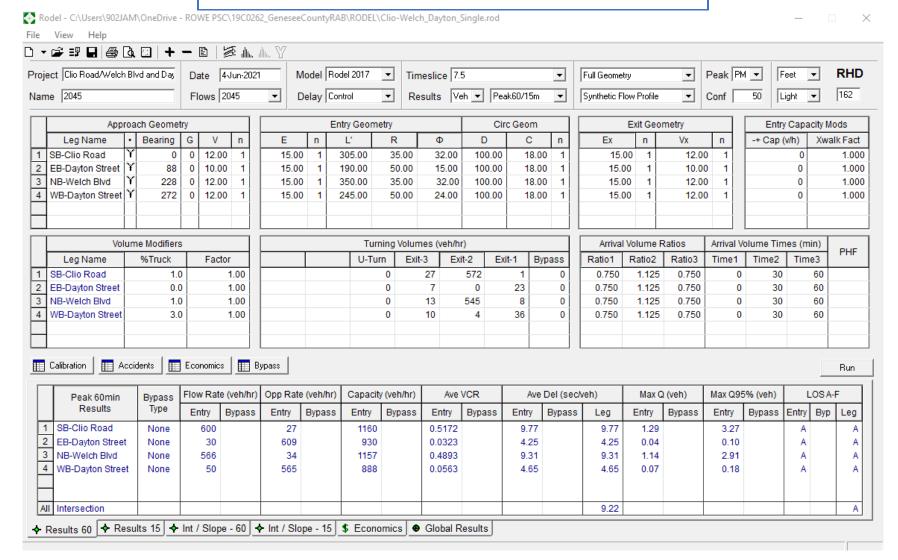
Network Summary

Network wide Queuing Penalty: 2

Clio Road/Welch Blvd. and W. Dayton Street – Single – AM Peak



Clio Road/Welch Blvd. and W. Dayton Street – Single – PM Peak



Intersection

Clio Road/Welch Blvd and West Dayton Street

Opinion of Probable Cost

By: Rowe PSC Date: 5/24/2021

PAY ITEM DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	AMOUNT
Mobilization (10%)	1	LSUM	\$90,000.00	\$90,000.00
Sidewalk, Rem	400	Syd	\$10.00	\$4,000.00
Pavt, Rem	7500	Syd	\$10.00	\$75,000.00
Curb and Gutter, Rem	2500	Ft	\$10.00	\$25,000.00
Embankment, CIP	3000	Cyd	\$15.00	\$45,000.00
Excavation, Earth	2200	Cyd	\$10.00	\$22,000.00
Aggregate Base	3400	Ton	\$21.00	\$71,400.00
Shoulder, CI II	0	Ton	\$25.00	\$0.00
Approach, Cl II	0	Ton	\$25.00	\$0.00
HMA, 4E10	1500	Ton	\$85.00	\$127,500.00
HMA Approach	3	Ton	\$50.00	\$150.00
Conc Pavt, Nonreinf, 9 inch	2300	Syd	\$45.00	\$103,500.00
Joint, Contraction, Cp	1850	Ft	\$10.00	\$18,500.00
Joint, Expansion, E2	330	Ft	\$25.00	\$8,250.00
Joint, Expansion, E3	830	Ft	\$15.00	\$12,450.00
Driveway, Nonreinf Conc, 9 inch	500	Syd	\$50.00	\$25,000.00
Curb and Gutter, Conc, Det B1	2240	Ft	\$25.00	\$56,000.00
Curb and Gutter, Conc, Det D1	210	Ft	\$25.00	\$5,250.00
Curb, Conc. Det E1	70	Ft	\$25.00	\$1,750.00
Driveway Opening, Conc, Det M	425	Ft	\$22.00	\$9,350.00
Detectable Warning Surface	40	Ft	\$40.00	\$1,600.00
Curb Ramp Opening, Conc	56	Ft	\$25.00	\$1,400.00
Sidewalk, Conc, 4 inch	4300	Sft	\$5.00	\$21,500.00
Sidewalk Ramp, Conc, 6 inch	780	Sft	\$10.00	\$7,800.00
Conc Pavt, Decorative Colored, 9 inch	3100	Sft	\$12.50	\$38,750.00
Turf Establishment, Performance	3000	Syd	\$5.00	\$15,000.00
MOT	1	LSUM	\$67,000.00	\$67,000.00
Pavement Markings	1	LSUM	\$15,000.00	\$15,000.00
Signing	1	LSUM	\$20,000.00	\$20,000.00
Drainage	1	LSUM	\$100,000	\$100,000.00
TOTAL				\$988,150

CONTINGENCY (20%) \$197,630.0

ESTIMATED TOTAL CONSTRUCTION COST (YEAR 2021) 3% ANNUAL INFLATION 2021 TO 2025 \$1,185,780.0 \$148,825.84

ESTIMATED TOTAL CONSTRUCTION COST (YEAR 2025)

\$1,334,605.84

* Notes

6" Agg base used for driveways and shoulders, 8" used for roadway, 12" used for islands (10% of total added) Asphalt shoulder gravel thickness was calculated using a 5" depth

Assumed 5" HMA thickness for approach and 8" thick for HMA roadway

- B1 curb was calculated by adding the splitter islands along with the roadways, minus the radius for the roundabout
- E1 curb was calculated for the inner roundabout curb
- E2 joint expansion was caclulated for the outer radius of the roundabout
- E3 joint expansion was caclulated using the radius of curves from the B/C of the roundabout and splitter
- D1 curb was calculated for the truck apron

Drainage includes enclosed storm for all curbed areas, spillways, and underdrain

MOT taken as approximately ~8% of total before mobilization; based on previous roundabout projects ROW acqusition costs not inlcuded

Joints assumed for central island/splitter islands

Pavement and curb removals include the assumption the south east island at the intersection will be removed.

NUMBER OF CRASHES OR INJURED PERSONS.

	YEAR 1	YEAR 2	YEAR 3	YEAR 4	YEAR 5
	2015	2016	2017	2018	2019
		700/			
Fatal and A-Injury Reduction Number of Crashes	%REDUCTION		Roundabout	0	1
Number of Crashes	0	0	0	0	1
A-Injured or Killed Persons	0	0	0	0	2
Minor Crash Reduction	%REDUCTION	57%	0	_	
Number of Crashes	0	0	0	0	15
	0	0	0	0	14 0
	U	U	U	U	U
	%REDUCTION	0%			
Number of Crashes	0	0	0	0	0
	0	0	0	0	0
	0	0	0	0	0
A-Injured or Killed Persons	0	0	0	0	0
	%REDUCTION	0%			
Number of Crashes	0	0	0	0	0
	0	0	0	0	0
	0	0	0	0	0
A-Injured or Killed Persons	0	0	0	0	0
	0/DEDUCTION	00/			
Number of Crashes	%REDUCTION 0	0% 0	0	0	0
Number of clusines	0	0	0	0	0
	0	0	0	0	0
A-Injured or Killed Persons	0	0	0	0	0
H of A introduce	_	2			
# of A-injuries: # of Fatalilties:	-		For reference	only; only; "Q" accou	ntc
# Of Fatalities.	<u>-</u>	<u> </u>	Torreference	only, Q accou	1103
			for the risk of	a fatality.	
PROJECT COST ES	TIMATE:	\$1,334,606	If unknown, er		
ADTb (before-vol	· · · · · · · · · · · · · · · · · · ·		You may chan		
ADTa (after-volur			default ADT va		
# OF YEARS OF D	_		3 to 5 years sh	ould be used.	
RATE OF INFLATION	ON:	2.50%			
AREA TYPE:		Urban	"Rural", "Urbar	n", or "Between	"

REMARKS:

Clio Road/Welch Blvd and Dayton Street Genesee County Roundabout Study 1505403, 1526401, 1532601 0, 1.1430708, 0.1498394 Roundabout

COMPUTED BENEFITS DERIVED THROUGH CRASH REDUCTION

TOR 2021

Date 9-Jul-21

Project: Clio Road/Welch Blvd and Dayton Street City/Twp. City of Flint
Prepared By: ROWE Professional Services Company County Genesee County

PR: 1505403, 1526401, 15326R MP Range: 0, 1.1430708, 0.1498394

The method of evaluating crash costs, used below, is given on page 67 of Roy Jorgensen's report of Highway Safety Improvement Criteria, 1966 edition. This same method is given in the Bureau of Public Roads IM21-3-67. In 1994 we have adapted the Q formula to blend Fatalities and A-injuries only. In the following analysis the costs provided by the National Safety Council are:

2019 NSC VALUES:

 Death
 \$1,659,000
 =FATCOST

 Disabling (A) injury:
 \$96,200
 =ACOST

 B-injury:
 \$27,800
 =BCOST

BTOTAL = ADTa / ADTb x [(Q x R1) + (BCOST x R2) + (PDOCOST x R3)]

WHERE:

PDO and/or Minor Injury Crash:

BTOTAL =	Total Benefit in Dollars Over Years Used	\$650,034
ADTa =	Average traffic volume after the improvement	1.1
ADTb =	Average traffic volume before the improvement	1.0
R1 =	Reduction in fatalities and A-Injuries Combined.	1.6
R2 =	Reduction in B-Injury crashes:	0.0
R3 =	Reduction in PDO and C-injury crashes:	8.0
Q =	$[FATCOST + ((I/F) \times INJCOST)] / [1 + (I/F)]$	
=	[1,659,000 + (6.10 x 96,200)] / [1 + 6.10]	\$316,400
	for AREA TYPE "Urban"	
I/F =		6.10

\$12,200 =PDOCOST

Q-Reference	Q	A-Inuries	Fatalities	I/F
RURAL	\$363,900	6,072	1,255	4.84
URBAN	\$316,400	9,902	1,624	6.10
BETWEEN	\$334,900	15,974	2,879	5.55

Data from Safety Programs Unit 5-Year Statewide Non-Trunkline Crash Figures Used. (*From 1-1-2015 Through 12-31-2019).

Time of Return (T.O.R.) is based on 5 years of data.

NOINFB =No-Inflation Annual Benefit=BTOTAL/years \$130,007

With an inflation rate of 2.50%

B=Annual Benefit=Present Value (with Inflation) \$166,420

C = Project Cost \$1,334,606

TOR=C/B=COST/ANNUAL BENEFIT=

8.02

D. Clio/Welch and Dayton

- 1. 2045 AM Peak Hour No Build
- 2. 2045 PM Peak Hour No Build
- 3. 2045 AM Peak Hour Roundabout
- 4. 2045 PM Peak Hour Roundabout